Planning Proposal for a Proposed Mixed Use Development

2 Farrow Road, Campbelltown

TRAFFIC AND PARKING ASSESSMENT REPORT

9 April 2020

Ref 20067



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1. INTRODUCTION

This report has been prepared to accompany a planning proposal for a mixed-use development to be located at 2 Farrow Road, Campbelltown (Figures 1 and 2).

The planning proposal seeks approval to increase the permissible building height and Floor Space Ratio (FSR) of the site to facilitate a mixed-use development comprising high-density residential apartment buildings with commercial, retail and community components.

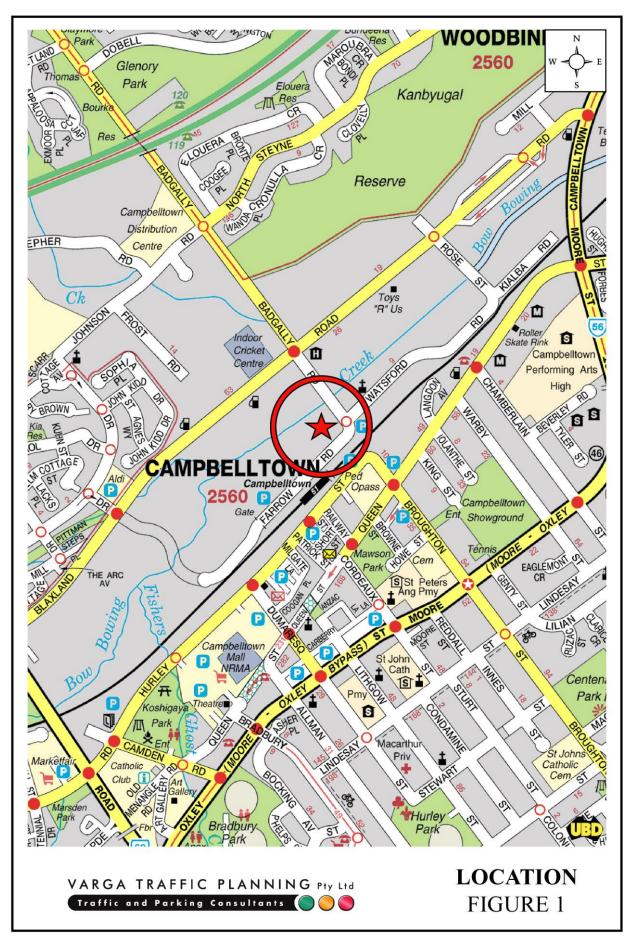
Off-street parking will be provided in basement car parking areas beneath each building, which will ultimately be designed to comply with the planning controls as well as the relevant Australian Standards.

Vehicular access to the site is envisaged to be provided via a new internal road network that will link with adjacent developments and provide connection to Farrow Road and Badgally Road.

Accordingly, this planning proposal has been prepared in accordance with the *Environmental Planning and Assessment Act 1979 (EP&A Act)* and the Department of Planning and Environment's -A Guide to Preparing Planning Proposals' (August 2016) to assess the traffic and parking implications of the planning proposal, and to that end this report:

- describes the site and provides details of the planning proposal
- reviews the road network in the vicinity of the site, and the traffic conditions on that road network
- estimates the traffic generation potential of the planning proposal, and assigns that traffic generation to the road network serving the site
- assesses the traffic implications of the planning proposal in terms of road network capacity

• reviews the off-street parking requirements of the planning proposal under statutory planning controls.





2. PLANNING PROPOSAL

Site

The subject site is located immediately north of the Campbelltown Railway Station, situated at the north-western corner of Badgally Road / Farrow Road intersection. The site has street frontages approximately 113 metres in length to Badgally Road, approximately 201 metres in length to Farrow Road, and occupies an area of approximately 28,122m².

The site is currently zoned DM – Deferred Matter and does not have any applicable height or FSR controls.

The site is currently occupied by a single storey warehouse building with a cumulative floor area in the order of 10,543m² with the remainder of the site generally providing landscape, and informal surface parking and loading.

A recent aerial image of the site and its surroundings is reproduced below.



Courtesy of SIX Maps 2020

Planning Proposal

The planning proposal seeks to ascertain the permissible building height and FSR of the site to facilitate a mixed-use development comprising high-density residential apartment buildings with commercial, retail and community components.

A total of seven mixed-use residential apartment buildings are envisaged, with height varying from 20 to 36 storeys, and FSR ranging from 4:1 to 5:1. It is estimated that the envisaged development on the site will provide:

- 1,565 residential apartments
- 4,481m² of commercial floor space
- 1,684m² of retail floor space, and
- 5,192m² of community space for local residents.

Off-street parking is envisaged to be provided in basement car parking areas beneath each building, which will ultimately be designed to comply with statutory planning controls as well as the relevant Australian Standards.

Vehicular access to the site is envisaged to be provided via a new internal road network that will link with adjacent developments and provide connection to Farrow Road and Badgally Road.

3. TRAFFIC ASSESSMENT

Road Hierarchy

The road hierarchy allocated to the road network in the vicinity of the site by the Roads and Maritime Services is illustrated on Figure 3.

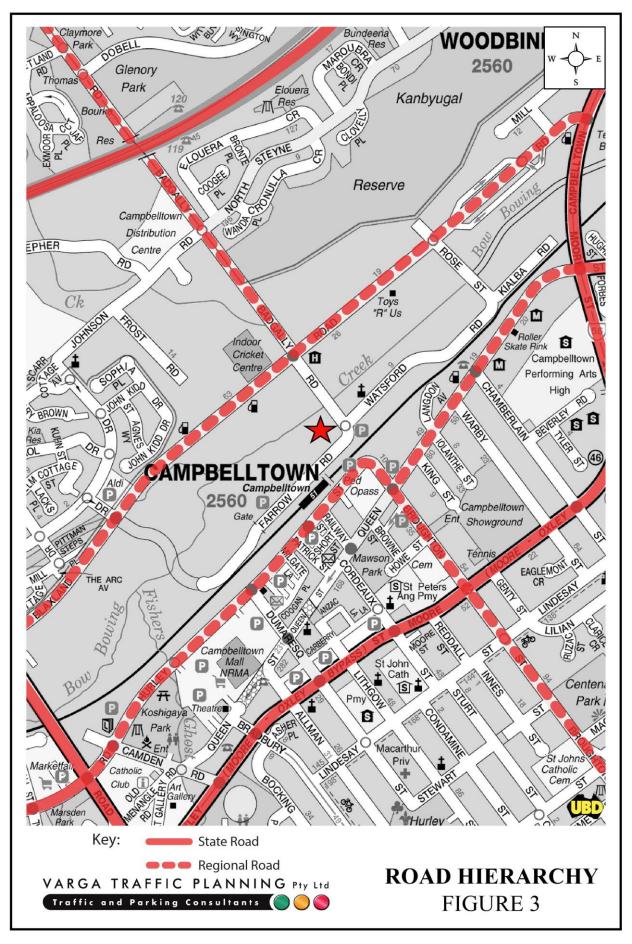
Oxley Street / Moore Street are classified by the RMS as a *State Road* which provides the key north-south road link through the Campbelltown area. It typically carries three traffic lanes in each direction in the vicinity of the site, with opposing traffic flows separated by a central median island.

Badgally Road (north of Blaxland Road) is classified by the RMS as a *Regional Road* and provides another north-south road link in the area, linking Blaxland Road and Camden Valley Way via Gregory Hills Drive. It carries multiple traffic lanes between Johnson Road and Blaxland Road, and generally one traffic lane in each direction elsewhere.

Blaxland Road is also classified by the RMS as a *Regional Road* and provides the key east-west road link in the area, linking Narellan Road and Campbelltown Road. It typically carries two traffic lanes in each direction in the vicinity of the site.

Badgally Road (along the site frontage) and Farrow Road are local, unclassified roads that are used to provide vehicular and pedestrian access to frontage properties. Kerbside parking is generally permitted in both of these roads.

It is understood that Campbelltown Council is currently investigating the provision of a bridge connecting Broughton Street and Badgally Road over the rail line, which is currently under internal review by the Government. It is noted however that the approach ramps required in Badgally Road for a bridge over the railway would extend past the site, such that the traffic generated by the site would not have any access to the bridge. As such, the proposed bridge, if it proceeds, would have minimal effect on the traffic flows entering and exiting the site.



Existing Traffic Controls

The existing traffic controls which apply to the road network in the vicinity of the site are illustrated on Figure 4. Key features of those traffic controls are:

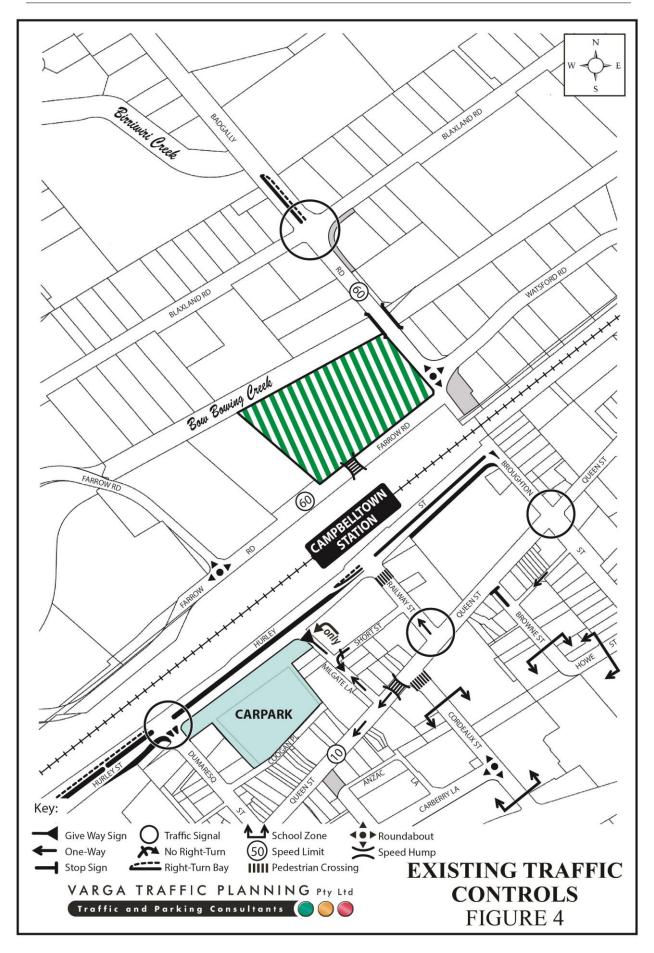
- a 60 km/h SPEED LIMIT which applies to Farrow Road and Badgally Road
- ROUNDABOUTS in Farrow Road where it intersects with Badgally Road / Watsford
 Road and where the road splits near the commuter car park
- TRAFFIC SIGNALS in Badgally Road where it intersects with Blaxland Road
- a NO RIGHT-TURN restriction in Badgally Road onto Blaxland Road
- a NO RIGHT-TURN restriction in Blaxland Road onto Badgally Road
- a RAISED PEDESTRIAN CROSSING towards the western end of the Farrow Road site frontage connecting to the Campbelltown Station.

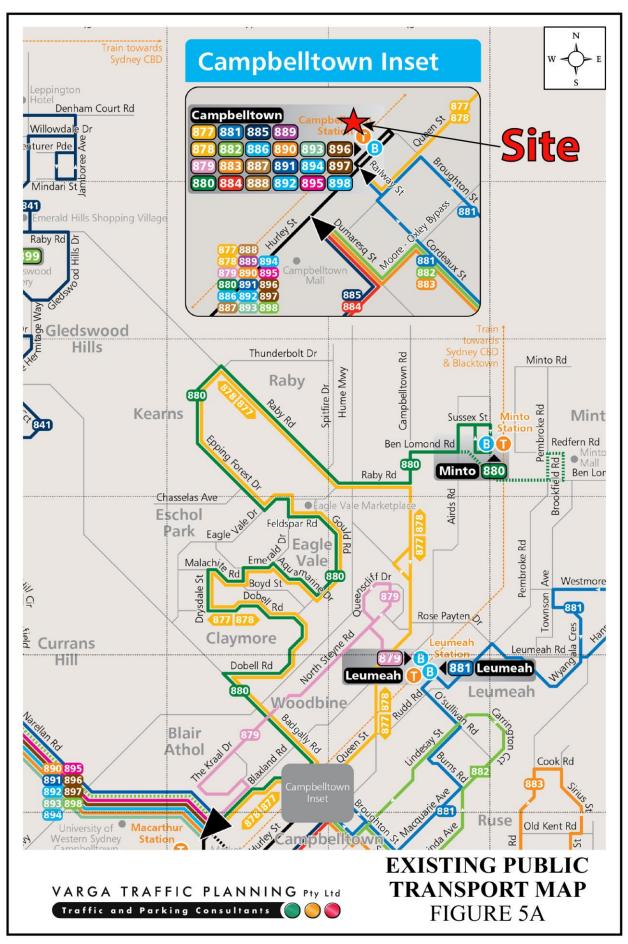
Existing Public Transport Services

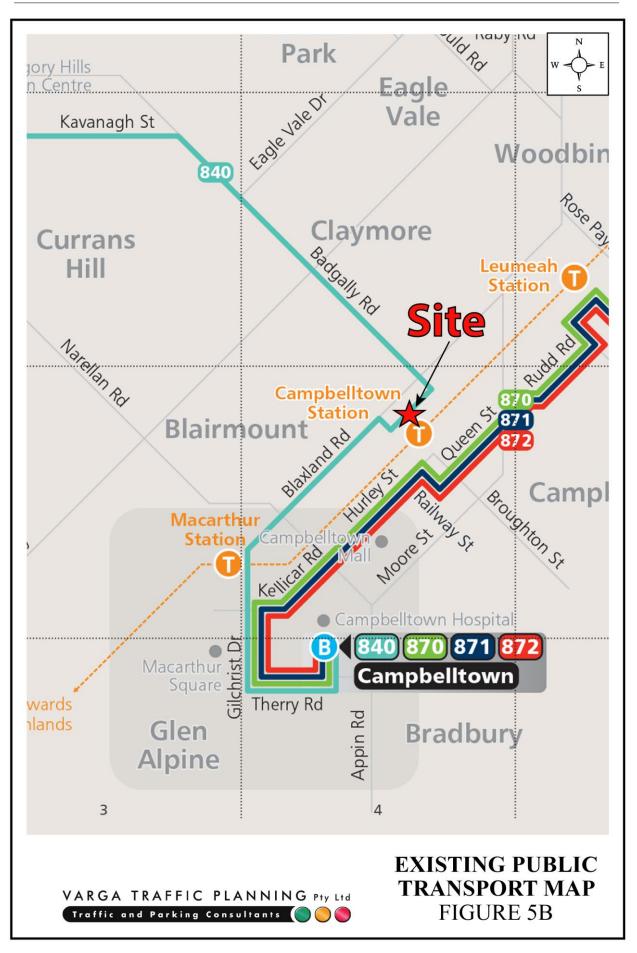
The existing public transport services available to the site are illustrated on Figure 5A and 5B.

The subject site is conveniently located immediately across the road from Campbelltown Railway Station, which services both the T8 Airport & South Line and also the intra-urban Southern Highlands Line (SHL).

The T8 Airport & South Line operates between Macarthur, Campbelltown, Turrella, Wolli Creek, International / Domestic Airports and the Sydney CBD, passing by major rail network interchanges at Glenfield and Sydenham. These services typically operate every 5-10 minutes during commuter peak periods and every 15 minutes at other times.







In addition to the train services, there are a significant number of bus routes currently operate in Farrow Road and the Campbelltown Railway Station bus terminal, services include:

- route 840 (Oran Park to Campbelltown)
- route 890 (Campbelltown to Harrington Park via Narellan Vale & Narellan)
- route 890C (Camden to Campbelltown via Narellan)
- route 891 (Mount Annan to Campbelltown via Currans Hill)
- route 892 (Campbelltown to Mount Annan via Narellan Vale)
- route 893 (Narellan to Campbelltown via Elderslie & Spring Farm)
- route 894 (Bridgewater Estate to Campbelltown via Camden & Narellan)
- route 894X (Bridgewater Estate to Campbelltown via Camden bypass)
- route 895 (Campbelltown to Camden South via Camden)
- route 896 (Campbelltown to Oran Park via Gregory Hills)
- route 897 (Smeaton Grange to Campbelltown)
- route 877 (Campbelltown to Kearns via Eagle Vale & Eschol Park)
- route 878 (Kearns to Campbelltown via Eschol Park)
- route 879 (Leumeah to Campbelltown via Blair Athol)
- route 880 (Minto to Campbelltown via Kearns & Eagle Vale)
- route 882 (Campbelltown to Leumeah South)
- route 883 (Campbelltown to Ruse)
- route 883K (Kentlyn to Campbelltown via Ruse)
- route 884 (Campbelltown to Airds)
- route 884W (Wedderburn to Campbelltown via Airds)
- route 885 (Campbelltown to Bradbury St & Helens Park North)
- route 885A (Campbelltown to Bradbury via Airds)
- route 886 (Campbelltown to Glen Alpine)
- route 887 (Wollongong to Campbelltown via Appin)
- route 888 (Campbelltown to St Helens Park via Ambarvale & Rosemeadow)
- route 889 (Menangle to Campbelltown via Menangle Park)
- route 900 (Picton to Campbelltown via Narellan).

Accordingly, it is clear that the site is readily accessible by public transport services, and is therefore ideally located to reduce reliance on private car usage and to facilitate sustainable transport habits.

Existing Traffic Conditions

An indication of the existing traffic conditions on the road network in the vicinity of the site is provided by peak period traffic surveys undertaken as part of this traffic study on Tuesday 25th February 2020 between 6:30am-9:30am and 3:30pm-6:0pm.

The traffic surveys were undertaken at Badgally Road / Farrow Road intersection, Badgally Road / Blaxland Road intersection, Blaxland Road / Farrow Road / Watsford Road intersection and the roundabout in Farrow Road near the commuter car park.

The results of the traffic surveys are reproduced in full in Appendix A and reveal that:

- two-way traffic flows in Blaxland Road are typically in the order of 2,000 to 3,000 vehicle trips per hour (vph) during commuter peak hours
- two-way traffic flows in Badgally Road are typically in the order of 1,100 to 1,400 vph during commuter peak hours north of Blaxland Road, and typically in the order of 300 vph south of Blaxland Road passing the site frontage
- two-way traffic flows in The Kraal Drive are typically in the order of 1,000 vph during commuter peak hours
- two-way traffic flows in Farrow Road are typically in the order of 250 to 550 vph during commuter peak hours
- two-way traffic flows in Watsford Road are typically in the order of 150 vph during commuter peak hours.

Projected Traffic Generation

An indication of the traffic generation potential of the planning proposal is provided by reference to the Roads and Maritime Services publication *Guide to Traffic Generating Developments, Section 3 - Landuse Traffic Generation (October 2002)* and the updated traffic generation rates in the RMS *Technical Direction (TDT 2013/04a)* document.

The *TDT 2013/04a* document specifies that it replaces those sections of the RMS *Guidelines* indicated, and that it must be followed when RMS is undertaken trip generation and/or parking demand assessments.

The RMS *Guidelines* and the updated *TDT 2013/04a* are based on extensive surveys of a wide range of land uses and nominate the following traffic generation rates which are applicable to the development proposal:

High Density Residential Flat Dwellings

AM: 0.19 peak hour vehicle trips per unit PM: 0.15 peak hour vehicle trips per unit

Commercial Office Blocks

AM: 1.6 peak hour vehicle trips per 100m² GFA
 PM: 1.2 peak hour vehicle trips per 100m² GFA

The RMS *Guidelines* do not nominate a traffic generation rate for small, local shops, referring only to major regional shopping centres incorporating supermarkets and department stores. For the purpose of this assessment therefore, the aforementioned traffic generation rates for *commercial premises* has been adopted in respect of the retail component of the development proposal.

Furthermore, the community component of the planning proposal is envisaged to provide leisure and recreational space for local residents living in the Campbelltown Centre that are within a walkable distance, and therefore could not be expected to attract or generate any appreciable amount of vehicular traffic.

Application of the above traffic generation rates and assumptions to the various components of the planning proposal yields a traffic generation potential of approximately 396 vph during the AM peak hour and approximately 309 vph during the PM peak hour as set out below:

Projected Total Future Traffic Generation Potential of the Site as a Consequence of the Planning Proposal on the Subject Site

	\mathbf{AM}	PM
Residential (1,565 apartments):	297.4 vph	234.8 vph
Commercial (4,481 m ²):	71.7 vph	53.8 vph
Retail (1,684m ²):	26.9 vph	20.2 vph
TOTAL TRAFFIC GENERATION POTENTIAL:	396.0 vph	308.8 vnh

That projected future level of traffic generation potential should however, be offset or *discounted* by the volume of traffic which could reasonably be expected to be generated by the existing development on the site, in order to determine the *nett increase* in traffic generation potential of the planning proposal.

Reference to the RMS *Guidelines* indicates that the existing 10,543m² warehouse has a traffic generation potential of approximately 105 vph during both the AM and PM peak hours.

Thus, the planning proposal could result in a *nett increase* in the traffic generation potential of the site of 291 vph in the AM peak hour, and 203 vph in the PM peak hour, as set out below:

Projected Nett Increase in Peak Hour Traffic Generation Potential as a Consequence of the Planning Proposal

	AM	PM
Projected Future Traffic Generation Potential:	396.0 vph	308.8 vph
Less Currently Approved Traffic Generation Potential:	-105.4 vph	-105.4 vph
NETT INCREASE IN TRAFFIC GENERATION POTENTIAL:	290.6 vph	203.4 vph

However, for the purposes a robust traffic assessment, it has been assumed that *all* of the projected future traffic flows of 396 vph in the AM peak hour and 309 vph in PM peak hour generated by both sites, will be *new* or *additional* to the existing traffic flows currently using the adjacent road network.

Traffic Implications - Road Network Capacity

The traffic implications of development proposals primarily concern the effects that any *additional* traffic flows may have on the operational performance of the nearby road network. Those effects can be assessed using the SIDRA program which is widely used by the RMS and many LGA's for this purpose. Criteria for evaluating the results of SIDRA analysis are reproduced in the following pages.

The results of the SIDRA capacity analysis of the key intersections are summarised below:

Badgally Road / The Kraal Drive / Farrow Road Intersection

- the intersection currently operates at *Level of Service "C"* and "D" under the existing traffic conditions with total average vehicle delays in the order of 36.2 to 47.1 seconds/vehicle
- under the projected future traffic demands expected to be generated by the planning proposal, the intersection will continue to operate satisfactorily at *existing Level of Service* with total average vehicle delays in the order of 37.9 to 48.9 seconds/vehicle

Badgally Road / Blaxland Road Intersection

- the intersection currently operates at *Level of Service "C"* under the existing traffic conditions with total average vehicle delays in the order of 34.8 to 39.3 seconds/vehicle
- under the projected future traffic demands expected to be generated by the planning proposal, the intersection will continue to operate satisfactorily at *existing Level of Service*, with total average vehicle delays in the order of 34.9 to 39.4 seconds/vehicle

Badgally Road / Farrow Road / Watsford Road Intersection

- the intersection currently operates at *Level of Service "A"* under the existing traffic conditions with total average vehicle delays in the order of 5.3 to 5.4 seconds/vehicle
- under the projected future traffic demands expected to be generated by the planning proposal, the intersection will continue to operate satisfactorily at *existing Level of Service*, with total average vehicle delays in the order of 6.3 to 7.0 seconds/vehicle

Farrow Road Roundabout (next to commuter car park)

- the intersection currently operates at *Level of Service "A"* under the existing traffic conditions with total average vehicle delays in the order of 6.5 to 7.3 seconds/vehicle
- under the projected future traffic demands expected to be generated by the planning proposal, the intersection will continue to operate satisfactorily at *existing Level of Service*, with total average vehicle delays in the order of 6.6 to 7.1 seconds/vehicle

SIDRA Modelling Results

T4	Van Indiaatana	Exis	sting	<u>Proj</u>	ected
Intersection	Key Indicators	\mathbf{AM}	PM	\mathbf{AM}	PM
Blaxland Rd,	LoS	С	D	С	D
The Kraal Dr &	DoS	0.843	0.914	0.859	0.917
Farrow Rd	Avg. Delay	36.2	47.1	37.9	48.9
Blaxland Rd	LoS	С	С	С	С
&	DoS	0.936	0.896	0.936	0.896
Badgally Rd	Avg. Delay	34.8	39.3	34.9	39.4
Badgally Rd,	LoS	A	A	A	A
Farrow Rd &	DoS	0.277	0.126	0.396	0.172
Watsford Rd	Avg. Delay	5.7	5.9	5.4	5.3
	LoS	A	A	A	A
Farrow Rd	DoS	0.433	0.248	0.454	0.256
Roundabout	Avg. Delay	6.5	7.3	6.6	7.1

LoS = Levels of Service

 $DoS = Degree \ of \ Saturation$

Delay = Total average vehicle delay (seconds per vehicle)

The detailed SIDRA movements summaries are reproduced in full in Appendix B.

In summary, the SIDRA capacity analysis demonstrates that the planning proposal will not have any unacceptable traffic implications whereby the surrounding intersections are expected to continue to operate satisfactorily at existing levels of service, and as such, there are no road improvements or intersection upgrades required as a consequence of the planning proposal.

Criteria for Interpreting Results of Sidra Analysis

1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good operation.	Good operation.
'B'	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
'C'	Satisfactory.	Satisfactory but accident study required.
'D'	Operating near capacity.	Near capacity and accident study required.
'E'	At capacity; at signals incidents will cause excessive	At capacity and requires other control mode.
	delays. Roundabouts require other control mode.	
'F'	Unsatisfactory and requires additional capacity.	Unsatisfactory and requires other control mode.

2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way and Stop Signs
A	less than 14	Good operation.	Good operation.
В	15 to 28	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
С	29 to 42	Satisfactory.	Satisfactory but accident study required.
D	43 to 56	Operating near capacity.	Near capacity and accident study required.
E	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode.	At capacity and requires other control mode.

3. Degree of Saturation (DS)

The DS is another measure of the operational performance of individual intersections.

For intersections controlled by traffic signals¹ both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a roundabout or GIVE WAY or STOP signs, satisfactory intersection operation is indicated by a DS of 0.8 or less.

The values of DS for intersections under traffic signal control are only valid for cycle length of 120 secs.

4. PARKING ASSESSMENT

Existing Kerbside Parking Restrictions

The existing kerbside parking restrictions which apply to the road network in the vicinity of the site are illustrated on Figure 6 and comprise:

- ¼ HOUR / 3 HOUR / ACESSIBLE PARKING restrictions in Farrow Road
- BUS ZONES at regular intervals in Farrow Road and Badgally Road
- 2 HOUR / UNRESTRICTED PARKING restrictions in Badgally Road.

Off-Street Parking Provisions

The off-street parking requirements applicable to the planning proposal are specified in *Campbelltown (Sustainable City) Development Control Plan 2015 – Volume 3 Deferred Areas DCP* document in the following terms:

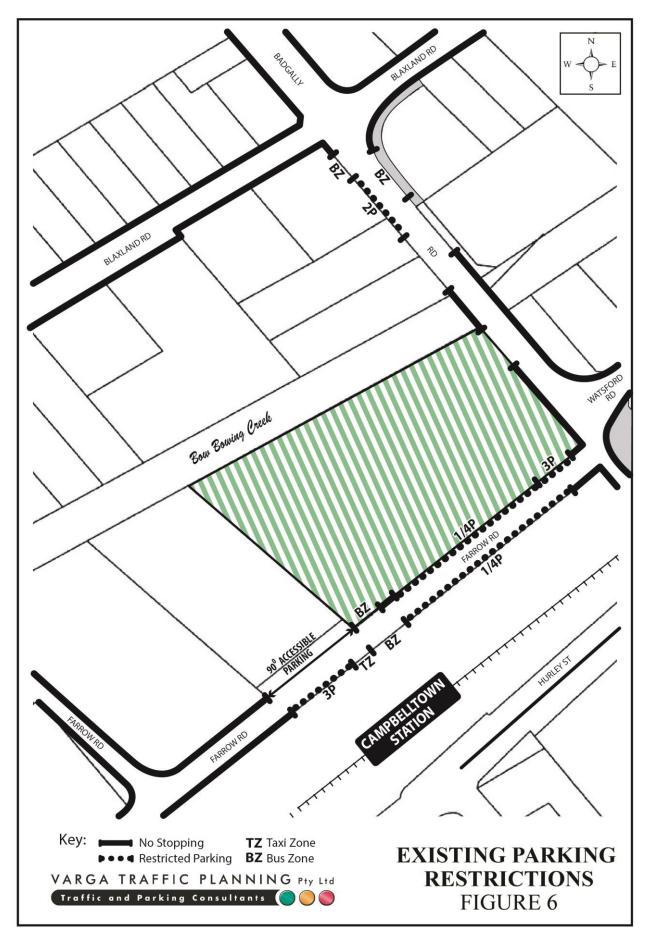
Clause 4.3 Residential Apartments

- Each dwelling shall be provided with a minimum of one car parking space, and
- An additional car parking space for every 4 dwellings (or part thereof), and
- An additional visitor car parking space for every 10 dwellings
- No required car parking space shall be in a stacked configuration
- Each development shall make provision for bicycle storage at a rate of 1 space per 5 dwellings within common property

Clause 4.4 Mixed Use Development

• In addition to residential car parking rates, the development shall provide one (1) car parking space per 25sqm of leasable floor space at ground level and one (1) car parking space per 35sqm of floor space at upper levels for all commercial / retail parts of the building

However, the subject site is located "within 800 metres" of a railway station in the Sydney metropolitan area (i.e. directly across from Campbelltown Railway Station), and therefore the residential component of the development is also subject to the parking requirements specified in the *State Environmental Planning Policy No 65 – Design Quality of Residential Flat Development (Amendment No 3), 2015* in the following terms:



30 Standards that cannot be used to refuse development consent or modification of development consent

- (1) If an application for the modification of a development consent or a development application for the carrying out of development to which this Policy applies satisfies the following design criteria, the consent authority must not refuse the application because of those matters:
 - a) if the car parking for the building will be equal to, or greater than, the recommended minimum amount of car parking specified in Part 3J of the Apartment Design Guide.

Reference is therefore made to the *Apartment Design Guide 2015*, Section 3J – Bicycle and Car Parking document which nominates the following car parking requirements:

Objective 3J-1

Car parking is provided based on proximity to public transport in metropolitan Sydney and centres in regional areas

For development in the following locations:

- on sites that are within 800 metres of a railway station or light rail stop in the Sydney Metropolitan
 Area; or
- on land zoned, and sites within 400 metres of land zoned, B3 Commercial Core, B4 Mixed Use or equivalent in a nominated regional centre

the minimum car parking requirements for residents and visitors is set out in the Guide to Traffic Generating Developments, or the car parking requirement prescribed by the relevant council, whichever is less.

The car parking needs for a development must be provided off street.

Comparison therefore needs to be drawn between the off-street car parking requirements for residential flat buildings outlined in the Council's *DCP 2015* and also the RMS *Guidelines* to determine the *lesser* requirement. The relevant car parking rates outlined in the RMS *Guidelines* are reproduced below:

RMS Guidelines - High Density Residential Flat Buildings

0.4 spaces per 1 bedroom unit

0.7 spaces per 2 bedroom unit

1.2 spaces per 3 bedroom unit

1 space per 7 units for visitor parking

Whilst the cumulative number of parking spaces to be provided as part of the planning proposal is not yet known, it is clear that the above parking requirements can be satisfied with the provision of a number of basement parking levels, based on the concept architectural plans which have been prepared for the purposes of the Planning Proposal.

The geometric design layout of the future car parking facilities will ultimately be designed to comply with the relevant requirements specified in the Standards Australia publication Parking Facilities Part 1 - Off-Street Car Parking AS2890.1:2004 and Parking Facilities Part 6 - Off-Street Parking for People with Disabilities AS2890.6.

Loading / Servicing Provisions

The proposed new development is expected to be serviced by a variety of light commercial vehicles and single rigid trucks.

The off-street loading / servicing facilities will ultimately be designed in accordance with Council requirements, and will be accommodate the swept turning path requirements of the largest vehicle to access the site, ensuring that they arrive and depart the site whilst travelling in forward gear at all times.

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5. CONCLUSION

Based on the analysis and discussions presented within this report, the following conclusions are made:

- the planning proposal ascertain the permissible building height and Floor Space Ratio (FSR) of the site to facilitate a mixed-use development comprising seven (7) mixed-use residential apartment buildings, with height varying from 20 to 36 storeys, and FSR ranging from 4:1 to 5:1. It is estimated that the envisaged development on the site will provide:
 - 1,565 residential apartments
 - 4,481m² of commercial floor space
 - 1,684m² of retail floor space, and
 - 5,192m² of community space for local residents.
- the SIDRA capacity analysis of the four (4) nearby key intersections located in the vicinity of the site indicate that:
 - the projected additional traffic flows as a consequence of the planning proposal will not have any adverse effects on the operational performance of the intersection, and
 - no road improvements or intersection upgrades would be required as a consequence of the planning proposal
- the future car parking, motorcycle, bicycle and loading facilities are capable of being provided in accordance with Council, *SEPP 65* and the relevant Australian Standards requirements, with detailed design and assessment to be completed at DA stage

 the future vehicular access arrangements will be designed in accordance with Council and RMS requirements.

It is therefore reasonable to conclude that the planning proposal will not have any unacceptable implications in terms of road network capacity or off-street parking/loading requirements.

APPENDIX A

TRAFFIC SURVEY DATA

Job No. : N5649

Client : Varga Traffic Planning

Suburb : Campbelltown

Location : 1. Badgally Rd / Blaxland Rd

Day/Date : Tue, 25th February 2020

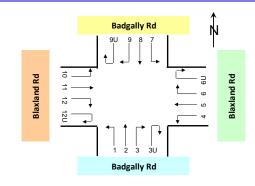
Weather : Fine

Description : Classified Intersection Count

: 15 mins Data

Class 1 Class 2

Classifications Lights Heavies





Approach						Badga	illy Rd						Blaxiand Rd											
Direction		irection Left Turr			irection (Through			irection Right Tur			irection : (U Turn)			irection Left Turr			irection Through			irection light Tur			rection ((U Turn)	
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
6:30 to 6:45	10	0	10	12	1	13	1	2	3	0	0	0	5	0	5	38	10	48	16	6	22	0	0	0
6:45 to 7:00	14	0	14	11	1	12	4	0	4	0	0	0	1	0	1	45	9	54	27	3	30	0	0	0
7:00 to 7:15	17	0	17	13	1	14	10	1	11	0	0	0	3	1	4	50	8	58	20	5	25	0	0	0
7:15 to 7:30	28	1	29	20	2	22	8	0	8	0	0	0	5	0	5	63	7	70	30	6	36	0	0	0
7:30 to 7:45	16	0	16	12	0	12	8	1	9	0	0	0	11	0	11	60	11	71	26	2	28	0	0	0
7:45 to 8:00	16	1	17	19	5	24	15	0	15	0	0	0	5	0	5	94	8	102	41	8	49	0	0	0
8:00 to 8:15	15	0	15	11	2	13	6	0	6	0	0	0	8	0	8	100	11	111	27	3	30	0	0	0
8:15 to 8:30	6	1	7	18	3	21	9	0	9	0	0	0	9	0	9	76	6	82	55	4	59	0	0	0
8:30 to 8:45	10	0	10	6	1	7	5	0	5	0	0	0	10	0	10	115	8	123	46	3	49	0	0	0
8:45 to 9:00	6	0	6	17	2	19	5	0	5	0	0	0	4	0	4	109	13	122	56	7	63	0	0	0
9:00 to 9:15	5	1	6	9	1	10	4	2	6	0	0	0	5	0	5	136	9	145	53	7	60	0	0	0
9:15 to 9:30	7	0	7	7	2	9	6	2	8	0	0	0	10	1	11	107	5	112	37	6	43	0	0	0
AM Totals	150	4	154	155	21	176	81	8	89	0	0	0	76	2	78	993	105	1,098	434	60	494	0	0	0
15:30 to 15:45	15	0	15	17	1	18	4	1	5	0	0	0	6	0	6	192	6	198	73	3	76	0	0	0
15:45 to 16:00	21	1	22	32	0	32	5	0	5	0	0	0	8	0	8	183	2	185	59	2	61	0	0	0
16:00 to 16:15	13	0	13	26	2	28	3	0	3	0	0	0	5	0	5	208	7	215	53	3	56	0	0	0
16:15 to 16:30	15	2	17	12	2	14	10	1	11	0	0	0	8	0	8	179	3	182	56	2	58	0	0	0
16:30 to 16:45	15	0	15	32	2	34	7	2	9	0	0	0	7	0	7	197	4	201	65	3	68	0	0	0
16:45 to 17:00	13	0	13	21	1	22	4	1	5	0	0	0	8	0	8	189	3	192	48	4	52	0	0	0
17:00 to 17:15	11	1	12	27	1	28	10	1	11	0	0	0	7	1	8	216	6	222	82	2	84	0	0	0
17:15 to 17:30	17	0	17	41	1	42	10	0	10	0	0	0	7	0	7	200	1	201	84	2	86	0	0	0
17:30 to 17:45	36	0	36	48	2	50	13	1	14	0	0	0	8	0	8	220	5	225	63	2	65	0	0	0
17:45 to 18:00	11	0	11	21	2	23	0	0	0	0	0	0	3	0	3	193	3	196	49	0	49	0	0	0
18:00 to 18:15	31	0	31	27	2	29	6	0	6	0	0	0	1	0	1	151	3	154	46	2	48	0	0	0
18:15 to 18:30	25	0	25	39	3	42	2	0	2	0	0	0	6	0	6	151	2	153	37	1	38	0	0	0
PM Totals	223	4	227	343	19	362	74	7	81	0	0	0	74	1	75	2,279	45	2,324	715	26	741	0	0	0

Approach		Badgally Rd Direction 7 Direction 8 Direction 9 Direction 9U																Blaxla	nd Rd					
Direction		irection Left Turn			irection Through			irection light Tur			rection ((U Turn)			irection Left Turr			irection Through			irection : Right Tur			rection 1 (U Turn)	
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
6:30 to 6:45	24	5	29	59	1	60	48	3	51	0	0	0	25	5	30	127	7	134	0	0	0	0	0	0
6:45 to 7:00	34	4	38	48	2	50	71	8	79	0	0	0	36	1	37	161	6	167	0	0	0	0	0	0
7:00 to 7:15	44	7	51	60	2	62	42	4	46	0	0	0	19	4	23	165	7	172	0	0	0	0	0	0
7:15 to 7:30	40	4	44	51	0	51	53	5	58	0	0	0	36	2	38	183	6	189	0	0	0	0	0	0
7:30 to 7:45	43	3	46	31	2	33	49	6	55	0	0	0	43	3	46	200	5	205	0	0	0	0	0	0
7:45 to 8:00	47	4	51	27	0	27	66	0	66	0	0	0	38	5	43	187	8	195	0	0	0	0	0	0
8:00 to 8:15	56	5	61	22	0	22	67	8	75	0	0	0	71	4	75	179	13	192	0	0	0	0	0	0
8:15 to 8:30	35	2	37	17	1	18	71	8	79	0	0	0	53	2	55	172	12	184	0	0	0	0	0	0
8:30 to 8:45	44	5	49	10	1	11	93	6	99	0	0	0	48	4	52	181	12	193	0	0	0	0	0	0
8:45 to 9:00	41	6	47	11	2	13	95	0	95	0	0	0	45	3	48	191	8	199	0	0	0	0	0	0
9:00 to 9:15	50	5	55	15	2	17	68	5	73	0	0	0	60	4	64	147	6	153	0	0	0	0	0	0
9:15 to 9:30	54	1	55	5	3	8	99	2	101	0	0	0	41	4	45	132	11	143	0	0	0	0	0	0
AM Totals	512	51	563	356	16	372	822	55	877	0	0	0	515	41	556	2,025	101	2,126	0	0	0	0	0	0
15:30 to 15:45	39	4	43	8	4	12	77	2	79	0	0	0	95	4	99	145	3	148	0	0	0	0	0	0
15:45 to 16:00	50	4	54	14	3	17	71	6	77	0	0	0	58	4	62	135	12	147	0	0	0	0	0	0
16:00 to 16:15	46	1	47	11	2	13	92	3	95	0	0	0	63	0	63	128	3	131	0	0	0	0	0	0
16:15 to 16:30	55	2	57	8	1	9	84	1	85	0	0	0	80	3	83	124	6	130	0	0	0	0	0	0
16:30 to 16:45	51	2	53	13	1	14	76	4	80	0	0	0	83	1	84	120	2	122	0	0	0	0	0	0
16:45 to 17:00	53	2	55	15	0	15	68	2	70	0	0	0	92	4	96	127	4	131	0	0	0	0	0	0
17:00 to 17:15	69	1	70	16	3	19	93	1	94	0	0	0	90	0	90	115	5	120	0	0	0	0	0	0
17:15 to 17:30	47	2	49	4	1	5	76	0	76	0	0	0	101	0	101	145	3	148	0	0	0	0	0	0
17:30 to 17:45	49	2	51	5	1	6	87	1	88	0	0	0	69	1	70	105	3	108	0	0	0	0	0	0
17:45 to 18:00	41	1	42	24	1	25	64	1	65	0	0	0	94	1	95	103	3	106	0	0	0	0	0	0
18:00 to 18:15	43	1	44	11	1	12	66	4	70	0	0	0	82	0	82	104	1	105	0	0	0	0	0	0
18:15 to 18:30	36	0	36	19	1	20	78	1	79	0	0	0	60	3	63	95	4	99	0	0	0	0	0	0
PM Totals	579	22	601	148	19	167	932	26	958	0	0	0	967	21	988	1,446	49	1,495	0	0	0	0	0	0

Job No. : N5649

Client : Varga Traffic Planning

Suburb : Campbelltown

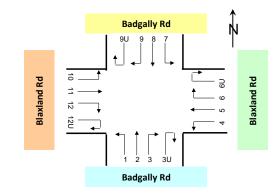
Location : 1. Badgally Rd / Blaxland Rd

Day/Date : Tue, 25th February 2020

Weather : Fine

Description : Classified Intersection Count

: Hourly Summary





Approach						Badga	lly Rd											Blaxla	ınd Rd					
Direction		irection Left Turn			irection Through			irection Right Tur			rection 3 (U Turn)			irection Left Turr			irection Through	-		irection light Tur			rection ((U Turn)	-
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
6:30 to 7:30	69	1	70	56	5	61	23	3	26	0	0	0	14	1	15	196	34	230	93	20	113	0	0	0
6:45 to 7:45	75	1	76	56	4	60	30	2	32	0	0	0	20	1	21	218	35	253	103	16	119	0	0	0
7:00 to 8:00	77	2	79	64	8	72	41	2	43	0	0	0	24	1	25	267	34	301	117	21	138	0	0	0
7:15 to 8:15	75	2	77	62	9	71	37	1	38	0	0	0	29	0	29	317	37	354	124	19	143	0	0	0
7:30 to 8:30	53	2	55	60	10	70	38	1	39	0	0	0	33	0	33	330	36	366	149	17	166	0	0	0
7:45 to 8:45	47	2	49	54	11	65	35	0	35	0	0	0	32	0	32	385	33	418	169	18	187	0	0	0
8:00 to 9:00	37	1	38	52	8	60	25	0	25	0	0	0	31	0	31	400	38	438	184	17	201	0	0	0
8:15 to 9:15	27	2	29	50	7	57	23	2	25	0	0	0	28	0	28	436	36	472	210	21	231	0	0	0
8:30 to 9:30	28	1	29	39	6	45	20	4	24	0	0	0	29	1	30	467	35	502	192	23	215	0	0	0
AM Totals	150	4	154	155	21	176	81	8	89	0	0	0	76	2	78	993	105	1,098	434	60	494	0	0	0
15:30 to 16:30	64	3	67	87	5	92	22	2	24	0	0	0	27	0	27	762	18	780	241	10	251	0	0	0
15:45 to 16:45	64	3	67	102	6	108	25	3	28	0	0	0	28	0	28	767	16	783	233	10	243	0	0	0
16:00 to 17:00	56	2	58	91	7	98	24	4	28	0	0	0	28	0	28	773	17	790	222	12	234	0	0	0
16:15 to 17:15	54	3	57	92	6	98	31	5	36	0	0	0	30	1	31	781	16	797	251	11	262	0	0	0
16:30 to 17:30	56	1	57	121	5	126	31	4	35	0	0	0	29	1	30	802	14	816	279	11	290	0	0	0
16:45 to 17:45	77	1	78	137	5	142	37	3	40	0	0	0	30	1	31	825	15	840	277	10	287	0	0	0
17:00 to 18:00	75	1	76	137	6	143	33	2	35	0	0	0	25	1	26	829	15	844	278	6	284	0	0	0
17:15 to 18:15	95	0	95	137	7	144	29	1	30	0	0	0	19	0	19	764	12	776	242	6	248	0	0	0
17:30 to 18:30	103	0	103	135	9	144	21	1	22	0	0	0	18	0	18	715	13	728	195	5	200	0	0	0
PM Totals	223	4	227	343	19	362	74	7	81	0	0	0	74	1	75	2,279	45	2,324	715	26	741	0	0	0

Approach		Badgally Rd											Blaxland Rd											
Direction		irection Left Turr			irection Through	-		irection ight Tur	-		rection 9 (U Turn)	9U		rection : Left Turn			rection 1 Through			rection 1			ection 1: (U Turn)	2U
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
6:30 to 7:30	142	20	162	218	5	223	214	20	234	0	0	0	116	12	128	636	26	662	0	0	0	0	0	0
6:45 to 7:45	161	18	179	190	6	196	215	23	238	0	0	0	134	10	144	709	24	733	0	0	0	0	0	0
7:00 to 8:00	174	18	192	169	4	173	210	15	225	0	0	0	136	14	150	735	26	761	0	0	0	0	0	0
7:15 to 8:15	186	16	202	131	2	133	235	19	254	0	0	0	188	14	202	749	32	781	0	0	0	0	0	0
7:30 to 8:30	181	14	195	97	3	100	253	22	275	0	0	0	205	14	219	738	38	776	0	0	0	0	0	0
7:45 to 8:45	182	16	198	76	2	78	297	22	319	0	0	0	210	15	225	719	45	764	0	0	0	0	0	0
8:00 to 9:00	176	18	194	60	4	64	326	22	348	0	0	0	217	13	230	723	45	768	0	0	0	0	0	0
8:15 to 9:15	170	18	188	53	6	59	327	19	346	0	0	0	206	13	219	691	38	729	0	0	0	0	0	0
8:30 to 9:30	189	17	206	41	8	49	355	13	368	0	0	0	194	15	209	651	37	688	0	0	0	0	0	0
AM Totals	512	51	563	356	16	372	822	55	877	0	0	0	515	41	556	2,025	101	2,126	0	0	0	0	0	0
15:30 to 16:30	190	11	201	41	10	51	324	12	336	0	0	0	296	11	307	532	24	556	0	0	0	0	0	0
15:45 to 16:45	202	9	211	46	7	53	323	14	337	0	0	0	284	8	292	507	23	530	0	0	0	0	0	0
16:00 to 17:00	205	7	212	47	4	51	320	10	330	0	0	0	318	8	326	499	15	514	0	0	0	0	0	0
16:15 to 17:15	228	7	235	52	5	57	321	8	329	0	0	0	345	8	353	486	17	503	0	0	0	0	0	0
16:30 to 17:30	220	7	227	48	5	53	313	7	320	0	0	0	366	5	371	507	14	521	0	0	0	0	0	0
16:45 to 17:45	218	7	225	40	5	45	324	4	328	0	0	0	352	5	357	492	15	507	0	0	0	0	0	0
17:00 to 18:00	206	6	212	49	6	55	320	3	323	0	0	0	354	2	356	468	14	482	0	0	0	0	0	0
17:15 to 18:15	180	6	186	44	4	48	293	6	299	0	0	0	346	2	348	457	10	467	0	0	0	0	0	0
17:30 to 18:30	169	4	173	59	4	63	295	7	302	0	0	0	305	5	310	407	11	418	0	0	0	0	0	0
PM Totals	579	22	601	148	19	167	932	26	958	0	0	0	967	21	988	1,446	49	1,495	0	0	0	0	0	0

Client : Varga Traffic Planning

Suburb : Campbelltown

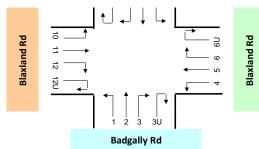
Location : 1. Badgally Rd / Blaxland Rd

Day/Date : Tue, 25th February 2020

Weather : Fine

Description : Classified Intersection Count

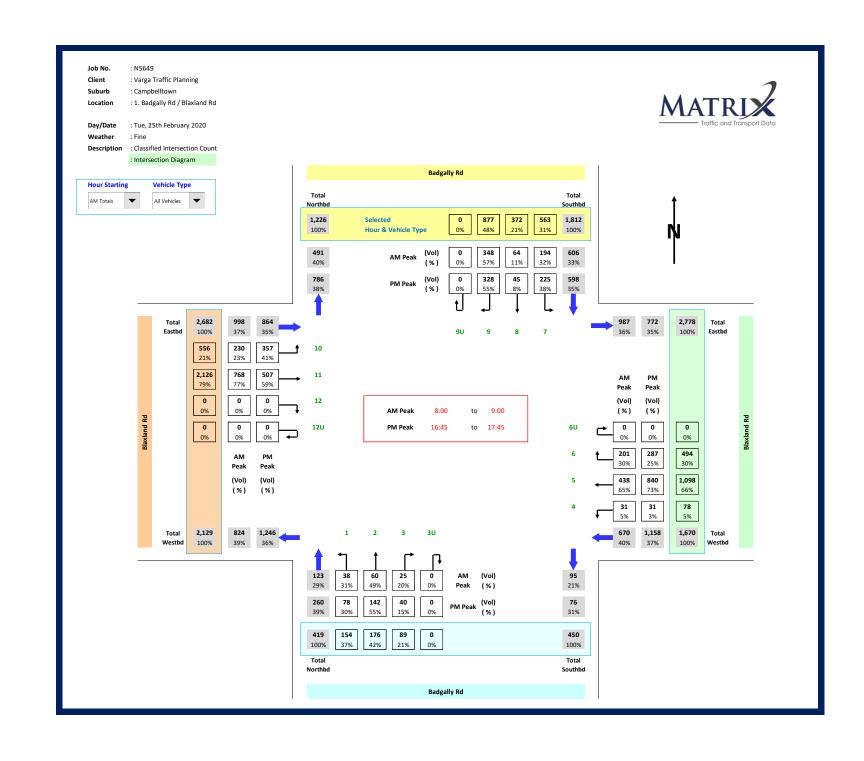
: Peak Hour Summary





	Approach			Ва	adgally F	₹d	ВІ	axland F	₹d	Ва	adgally F	Rd	ВІ	Rd	Total	
	Time Period			Lights	Lights Heavies Total		± 0		Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand T
AM	8:00	to	9:00	114	9	123	615	55	670	562	44	606	940	58	998	2,397
PM	16:45	to	17:45	251	9	260	1,132	26	1,158	582	16	598	844	20	864	2,880

Approach		Ва	adgally I	Rd	ВІ	axland I	Rd	Ва	adgally I	Rd	ВІ	otal			
Time Period			Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand Total
6:30	to	7:30	148	9	157	303	55	358	574	45	619	752	38	790	1,924
6:45	to	7:45	161	7	168	341	52	393	566	47	613	843	34	877	2,051
7:00	to	8:00	182	12	194	408	56	464	553	37	590	871	40	911	2,159
7:15	to	8:15	174	12	186	470	56	526	552	37	589	937	46	983	2,284
7:30	to	8:30	151	13	164	512	53	565	531	39	570	943	52	995	2,294
7:45	to	8:45	136	13	149	586	51	637	555	40	595	929	60	989	2,370
8:00	to	9:00	114	9	123	615	55	670	562	44	606	940	58	998	2,397
8:15	to	9:15	100	11	111	674	57	731	550	43	593	897	51	948	2,383
8:30	to	9:30	87	11	98	688	59	747	585	38	623	845	52	897	2,365
ΑN	/I Tot	als	386	33	419	1,503	167	1,670	1,690	122	1,812	2,540	142	2,682	6,583
15:30	to	16:30	173	10	183	1,030	28	1,058	555	33	588	828	35	863	2,692
15:45	to	16:45	191	12	203	1,028	26	1,054	571	30	601	791	31	822	2,680
16:00	to	17:00	171	13	184	1,023	29	1,052	572	21	593	817	23	840	2,669
16:15	to	17:15	177	14	191	1,062	28	1,090	601	20	621	831	25	856	2,758
16:30	to	17:30	208	10	218	1,110	26	1,136	581	19	600	873	19	892	2,846
16:45	to	17:45	251	9	260	1,132	26	1,158	582	16	598	844	20	864	2,880
17:00	to	18:00	245	9	254	1,132	22	1,154	575	15	590	822	16	838	2,836
17:15	to	18:15	261	8	269	1,025	18	1,043	517	16	533	803	12	815	2,660
17:30	to	18:30	259	10	269	928	18	946	523	15	538	712	16	728	2,481
PM Totals		640	30	670	3,068	72	3,140	1,659	67	1,726	2,413	70	2,483	8,019	



Job No. : N5649

Client : Varga Traffic Planning

Suburb : Campbelltown

Location : 2. Blaxland Rd / The Kraal Dr / Farrow Rd

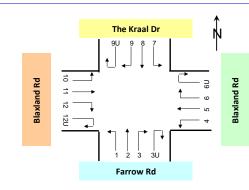
Day/Date : Tue, 25th February 2020

Weather : Fine

Description : Classified Intersection Count

: 15 mins Data

Class 1 Class 2
Classifications Lights Heavies





Approach	Farrow Rd											Blaxland Rd												
Direction		Direction 1 (Left Turn)			Direction 2 (Through)			irection Right Tur		Direction 3U (U Turn)			Direction 4 (Left Turn)				irection Through		Direction 6 (Right Turn)			Direction 6U (U Turn)		
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
6:30 to 6:45	7	0	7	8	0	8	1	3	4	0	0	0	3	1	4	101	11	112	3	1	4	0	0	0
6:45 to 7:00	22	0	22	9	0	9	3	0	3	0	0	0	2	0	2	112	21	133	3	0	3	0	0	0
7:00 to 7:15	17	2	19	6	1	7	5	0	5	0	0	0	4	0	4	94	10	104	6	0	6	0	0	0
7:15 to 7:30	18	1	19	4	0	4	1	0	1	0	0	0	3	3	6	126	10	136	6	1	7	0	0	0
7:30 to 7:45	11	0	11	9	0	9	4	1	5	0	0	0	1	1	2	111	15	126	4	0	4	0	0	0
7:45 to 8:00	14	2	16	7	1	8	2	1	3	0	0	0	4	0	4	156	6	162	8	0	8	0	0	0
8:00 to 8:15	17	1	18	10	0	10	0	0	0	0	0	0	2	2	4	158	14	172	9	0	9	0	0	0
8:15 to 8:30	11	2	13	3	1	4	1	0	1	0	0	0	5	4	9	144	11	155	5	1	6	0	0	0
8:30 to 8:45	12	1	13	4	1	5	0	1	1	0	0	0	3	1	4	194	10	204	5	2	7	0	0	0
8:45 to 9:00	13	2	15	0	0	0	1	1	2	0	0	0	0	1	1	209	17	226	8	0	8	0	0	0
9:00 to 9:15	10	6	16	2	0	2	4	2	6	0	0	0	1	2	3	194	12	206	13	0	13	0	0	0
9:15 to 9:30	10	3	13	2	1	3	0	0	0	0	0	0	3	0	3	189	6	195	14	1	15	1	0	1
AM Totals	162	20	182	64	5	69	22	9	31	0	0	0	31	15	46	1,788	143	1,931	84	6	90	1	0	1
15:30 to 15:45	32	2	34	9	0	9	1	3	4	0	0	0	2	1	3	253	8	261	22	1	23	0	0	0
15:45 to 16:00	25	2	27	11	0	11	4	0	4	0	0	0	2	3	5	250	7	257	17	0	17	0	0	0
16:00 to 16:15	43	3	46	13	0	13	5	0	5	0	0	0	2	0	2	291	8	299	18	2	20	0	0	0
16:15 to 16:30	37	2	39	8	0	8	1	2	3	0	0	0	2	0	2	252	6	258	12	0	12	0	0	0
16:30 to 16:45	53	2	55	9	0	9	3	0	3	0	0	0	1	1	2	289	5	294	23	0	23	1	0	1
16:45 to 17:00	58	0	58	6	0	6	4	0	4	0	0	0	3	0	3	226	6	232	33	0	33	0	0	0
17:00 to 17:15	92	3	95	15	1	16	1	1	2	0	0	0	1	0	1	277	5	282	26	1	27	0	0	0
17:15 to 17:30	68	1	69	15	0	15	4	1	5	0	0	0	6	0	6	275	1	276	31	0	31	0	0	0
17:30 to 17:45	129	1	130	16	0	16	2	0	2	0	0	0	3	1	4	304	4	308	23	1	24	0	0	0
17:45 to 18:00	52	1	53	10	1	11	1	0	1	0	0	0	1	0	1	242	4	246	22	1	23	0	0	0
18:00 to 18:15	97	1	98	11	0	11	7	0	7	0	0	0	2	0	2	219	1	220	25	0	25	0	0	0
18:15 to 18:30	63	3	66	12	0	12	4	0	4	0	0	0	4	1	5	247	4	251	20	0	20	0	0	0
PM Totals	749	21	770	135	2	137	37	7	44	0	0	0	29	7	36	3,125	59	3,184	272	6	278	1	0	1

Approach	The Kraal Dr											Blaxland Rd												
Direction	Direction 7 (Left Turn)			Direction 8 (Through)			Direction 9 (Right Turn)			Direction 9U (U Turn)			Direction 10 (Left Turn)			Direction 11 (Through)			Direction 12 (Right Turn)			Direction 12U (U Turn)		
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
6:30 to 6:45	13	1	14	42	0	42	18	2	20	0	0	0	69	2	71	156	8	164	148	2	150	0	0	0
6:45 to 7:00	10	1	11	30	0	30	59	5	64	0	0	0	62	3	65	197	10	207	151	0	151	0	0	0
7:00 to 7:15	15	0	15	14	1	15	38	4	42	0	0	0	69	2	71	158	8	166	105	3	108	0	0	0
7:15 to 7:30	13	2	15	14	1	15	36	1	37	0	0	0	71	2	73	238	8	246	96	0	96	0	0	0
7:30 to 7:45	13	0	13	17	0	17	48	3	51	0	0	0	91	2	93	237	5	242	69	2	71	0	0	0
7:45 to 8:00	19	0	19	8	1	9	82	1	83	0	0	0	117	3	120	262	10	272	40	4	44	0	0	0
8:00 to 8:15	19	0	19	11	0	11	108	1	109	0	0	0	120	2	122	221	14	235	42	1	43	2	0	2
8:15 to 8:30	15	0	15	2	2	4	101	3	104	0	0	0	101	4	105	243	19	262	33	1	34	1	0	1
8:30 to 8:45	16	1	17	5	0	5	102	3	105	0	0	0	104	2	106	193	14	207	35	2	37	1	0	1
8:45 to 9:00	17	2	19	3	0	3	85	2	87	0	0	0	86	4	90	248	9	257	32	3	35	2	0	2
9:00 to 9:15	9	0	9	5	0	5	81	1	82	0	0	0	99	6	105	181	8	189	17	0	17	0	0	0
9:15 to 9:30	22	0	22	6	0	6	73	1	74	0	0	0	80	1	81	158	12	170	19	2	21	0	0	0
AM Totals	181	7	188	157	5	162	831	27	858	0	0	0	1,069	33	1,102	2,492	125	2,617	787	20	807	6	0	6
15:30 to 15:45	12	0	12	5	0	5	104	3	107	0	0	0	117	1	118	215	3	218	19	2	21	0	0	0
15:45 to 16:00	13	0	13	8	0	8	81	2	83	0	0	0	118	1	119	185	11	196	25	0	25	1	0	1
16:00 to 16:15	21	0	21	6	0	6	118	3	121	0	0	0	85	1	86	167	3	170	20	4	24	0	0	0
16:15 to 16:30	11	1	12	12	0	12	94	3	97	0	0	0	102	3	105	182	6	188	19	4	23	0	0	0
16:30 to 16:45	14	1	15	0	0	0	86	0	86	0	0	0	97	1	98	193	1	194	18	1	19	0	0	0
16:45 to 17:00	22	0	22	5	0	5	81	3	84	0	0	0	95	1	96	177	11	188	16	3	19	0	0	0
17:00 to 17:15	16	0	16	5	0	5	95	1	96	0	0	0	101	2	103	183	3	186	20	3	23	0	0	0
17:15 to 17:30	39	0	39	6	0	6	84	1	85	0	0	0	99	1	100	196	5	201	26	1	27	0	0	0
17:30 to 17:45	11	0	11	7	0	7	83	1	84	0	0	0	93	1	94	150	5	155	21	2	23	0	0	0
17:45 to 18:00	18	1	19	9	0	9	71	1	72	0	0	0	124	0	124	171	4	175	23	2	25	0	0	0
18:00 to 18:15	16	0	16	10	0	10	79	1	80	0	0	0	76	1	77	167	2	169	27	2	29	0	0	0
18:15 to 18:30	14	0	14	6	0	6	78	0	78	0	0	0	79	0	79	138	5	143	26	1	27	0	0	0
PM Totals	207	3	210	79	0	79	1,054	19	1,073	0	0	0	1,186	13	1,199	2,124	59	2,183	260	25	285	1	0	1

Client : Varga Traffic Planning

Suburb : Campbelltown

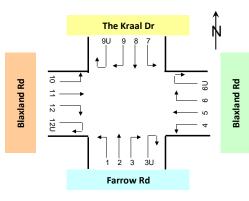
Location : 2. Blaxland Rd / The Kraal Dr / Farrow Rd

Day/Date : Tue, 25th February 2020

Weather : Fine

Description : Classified Intersection Count

: Hourly Summary





Approach						Farro	w Rd											Blaxla	nd Rd					
Direction		irection Left Turn			irection (Through			irection Right Tur			irection ((U Turn)	BU		irection Left Turr			irection Through			irection light Tur			rection ((U Turn)	
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
6:30 to 7:30	64	3	67	27	1	28	10	3	13	0	0	0	12	4	16	433	52	485	18	2	20	0	0	0
6:45 to 7:45	68	3	71	28	1	29	13	1	14	0	0	0	10	4	14	443	56	499	19	1	20	0	0	0
7:00 to 8:00	60	5	65	26	2	28	12	2	14	0	0	0	12	4	16	487	41	528	24	1	25	0	0	0
7:15 to 8:15	60	4	64	30	1	31	7	2	9	0	0	0	10	6	16	551	45	596	27	1	28	0	0	0
7:30 to 8:30	53	5	58	29	2	31	7	2	9	0	0	0	12	7	19	569	46	615	26	1	27	0	0	0
7:45 to 8:45	54	6	60	24	3	27	3	2	5	0	0	0	14	7	21	652	41	693	27	3	30	0	0	0
8:00 to 9:00	53	6	59	17	2	19	2	2	4	0	0	0	10	8	18	705	52	757	27	3	30	0	0	0
8:15 to 9:15	46	11	57	9	2	11	6	4	10	0	0	0	9	8	17	741	50	791	31	3	34	0	0	0
8:30 to 9:30	45	12	57	8	2	10	5	4	9	0	0	0	7	4	11	786	45	831	40	3	43	1	0	1
AM Totals	162	20	182	64	5	69	22	9	31	0	0	0	31	15	46	1,788	143	1,931	84	6	90	1	0	1
15:30 to 16:30	137	9	146	41	0	41	11	5	16	0	0	0	8	4	12	1,046	29	1,075	69	3	72	0	0	0
15:45 to 16:45	158	9	167	41	0	41	13	2	15	0	0	0	7	4	11	1,082	26	1,108	70	2	72	1	0	1
16:00 to 17:00	191	7	198	36	0	36	13	2	15	0	0	0	8	1	9	1,058	25	1,083	86	2	88	1	0	1
16:15 to 17:15	240	7	247	38	1	39	9	3	12	0	0	0	7	1	8	1,044	22	1,066	94	1	95	1	0	1
16:30 to 17:30	271	6	277	45	1	46	12	2	14	0	0	0	11	1	12	1,067	17	1,084	113	1	114	1	0	1
16:45 to 17:45	347	5	352	52	1	53	11	2	13	0	0	0	13	1	14	1,082	16	1,098	113	2	115	0	0	0
17:00 to 18:00	341	6	347	56	2	58	8	2	10	0	0	0	11	1	12	1,098	14	1,112	102	3	105	0	0	0
17:15 to 18:15	346	4	350	52	1	53	14	1	15	0	0	0	12	1	13	1,040	10	1,050	101	2	103	0	0	0
17:30 to 18:30	341	6	347	49	1	50	14	0	14	0	0	0	10	2	12	1,012	13	1,025	90	2	92	0	0	0
PM Totals	749	21	770	135	2	137	37	7	44	0	0	0	29	7	36	3,125	59	3,184	272	6	278	1	0	1

Approach						The Kı	raal Dr											Blaxla	nd Rd					
Direction	_	irection Left Turn	-		irection Through	-	_	irection light Tur	-		rection ((U Turn)			rection : Left Turr			rection : Through			rection :			ection 1 (U Turn)	2U
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
6:30 to 7:30	51	4	55	100	2	102	151	12	163	0	0	0	271	9	280	749	34	783	500	5	505	0	0	0
6:45 to 7:45	51	3	54	75	2	77	181	13	194	0	0	0	293	9	302	830	31	861	421	5	426	0	0	0
7:00 to 8:00	60	2	62	53	3	56	204	9	213	0	0	0	348	9	357	895	31	926	310	9	319	0	0	0
7:15 to 8:15	64	2	66	50	2	52	274	6	280	0	0	0	399	9	408	958	37	995	247	7	254	2	0	2
7:30 to 8:30	66	0	66	38	3	41	339	8	347	0	0	0	429	11	440	963	48	1,011	184	8	192	3	0	3
7:45 to 8:45	69	1	70	26	3	29	393	8	401	0	0	0	442	11	453	919	57	976	150	8	158	4	0	4
8:00 to 9:00	67	3	70	21	2	23	396	9	405	0	0	0	411	12	423	905	56	961	142	7	149	6	0	6
8:15 to 9:15	57	3	60	15	2	17	369	9	378	0	0	0	390	16	406	865	50	915	117	6	123	4	0	4
8:30 to 9:30	64	3	67	19	0	19	341	7	348	0	0	0	369	13	382	780	43	823	103	7	110	3	0	3
AM Totals	181	7	188	157	5	162	831	27	858	0	0	0	1,069	33	1,102	2,492	125	2,617	787	20	807	6	0	6
15:30 to 16:30	57	1	58	31	0	31	397	11	408	0	0	0	422	6	428	749	23	772	83	10	93	1	0	1
15:45 to 16:45	59	2	61	26	0	26	379	8	387	0	0	0	402	6	408	727	21	748	82	9	91	1	0	1
16:00 to 17:00	68	2	70	23	0	23	379	9	388	0	0	0	379	6	385	719	21	740	73	12	85	0	0	0
16:15 to 17:15	63	2	65	22	0	22	356	7	363	0	0	0	395	7	402	735	21	756	73	11	84	0	0	0
16:30 to 17:30	91	1	92	16	0	16	346	5	351	0	0	0	392	5	397	749	20	769	80	8	88	0	0	0
16:45 to 17:45	88	0	88	23	0	23	343	6	349	0	0	0	388	5	393	706	24	730	83	9	92	0	0	0
17:00 to 18:00	84	1	85	27	0	27	333	4	337	0	0	0	417	4	421	700	17	717	90	8	98	0	0	0
17:15 to 18:15	84	1	85	32	0	32	317	4	321	0	0	0	392	3	395	684	16	700	97	7	104	0	0	0
17:30 to 18:30	59	1	60	32	0	32	311	3	314	0	0	0	372	2	374	626	16	642	97	7	104	0	0	0
PM Totals	207	3	210	79	0	79	1,054	19	1,073	0	0	0	1,186	13	1,199	2,124	59	2,183	260	25	285	1	0	1

Client : Varga Traffic Planning

Suburb : Campbelltown

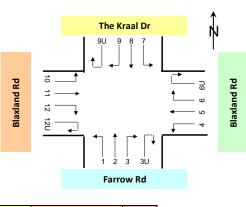
Location : 2. Blaxland Rd / The Kraal Dr / Farrow Rd

Day/Date : Tue, 25th February 2020

Weather : Fine

Description : Classified Intersection Count

: Peak Hour Summary

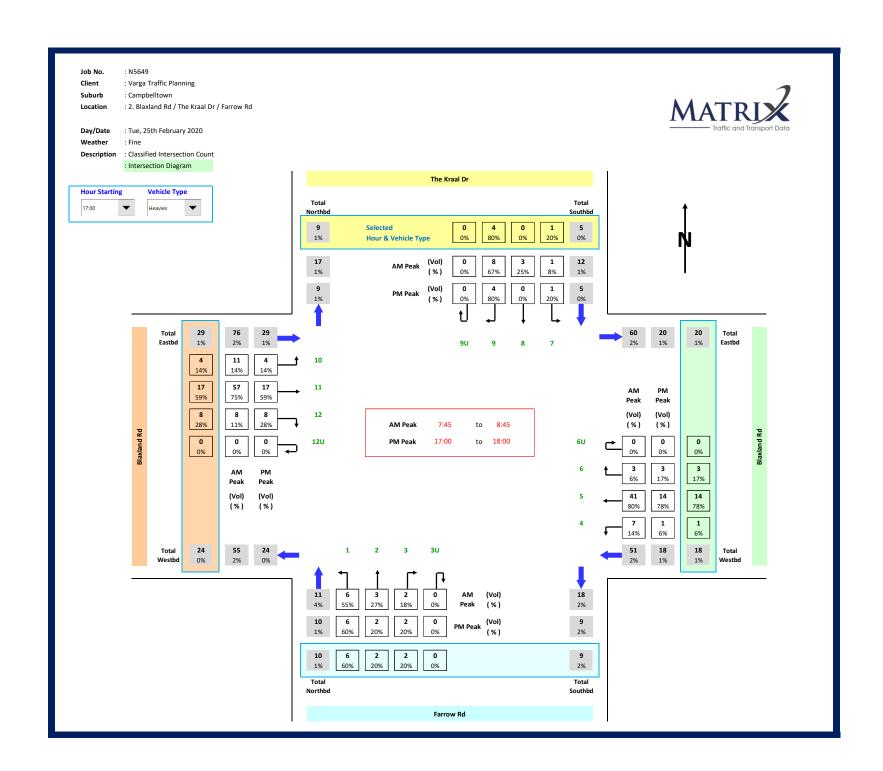




	Ap	proa	ich	F	arrow R	d	ВІ	axland I	Rd	Th	e Kraal	Dr	ВІ	axland I	Rd	otal
	Tim	e Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand T
M	7:45	to	8:45	81	11	92	693	51	744	488	12	500	1,515	76	1,591	2,927
М	17:00	to	18:00	405	10	415	1,211	18	1,229	444	5	449	1,207	29	1,236	3,329

Арі	proa	ich	F	arrow R	d	ВІ	axland I	Rd	Th	e Kraal	Dr	ВІ	axland I	₹d	otal
Time	e Pei	riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand Total
6:30	to	7:30	101	7	108	463	58	521	302	18	320	1,520	48	1,568	2,517
6:45	to	7:45	109	5	114	472	61	533	307	18	325	1,544	45	1,589	2,561
7:00	to	8:00	98	9	107	523	46	569	317	14	331	1,553	49	1,602	2,609
7:15	to	8:15	97	7	104	588	52	640	388	10	398	1,606	53	1,659	2,801
7:30	to	8:30	89	9	98	607	54	661	443	11	454	1,579	67	1,646	2,859
7:45	to	8:45	81	11	92	693	51	744	488	12	500	1,515	76	1,591	2,927
8:00	to	9:00	72	10	82	742	63	805	484	14	498	1,464	75	1,539	2,924
8:15	to	9:15	61	17	78	781	61	842	441	14	455	1,376	72	1,448	2,823
8:30	to	9:30	58	18	76	834	52	886	424	10	434	1,255	63	1,318	2,714
AM	Tot	als	248	34	282	1,904	164	2,068	1,169	39	1,208	4,354	178	4,532	8,090
15:30	to	16:30	189	14	203	1,123	36	1,159	485	12	497	1,255	39	1,294	3,153
15:45	to	16:45	212	11	223	1,160	32	1,192	464	10	474	1,212	36	1,248	3,137
16:00	to	17:00	240	9	249	1,153	28	1,181	470	11	481	1,171	39	1,210	3,121
16:15	to	17:15	287	11	298	1,146	24	1,170	441	9	450	1,203	39	1,242	3,160
16:30	to	17:30	328	9	337	1,192	19	1,211	453	6	459	1,221	33	1,254	3,261
16:45	to	17:45	410	8	418	1,208	19	1,227	454	6	460	1,177	38	1,215	3,320
17:00	to	18:00	405	10	415	1,211	18	1,229	444	5	449	1,207	29	1,236	3,329
17:15	to	18:15	412	6	418	1,153	13	1,166	433	5	438	1,173	26	1,199	3,221
17:30	to	18:30	404	7	411	1,112	17	1,129	402	4	406	1,095	25	1,120	3,066
PM	Tot	als	921	30	951	3,427	72	3,499	1,340	22	1,362	3,571	97	3,668	9,480

AM



Client : Varga Traffic Planning

Suburb : Campbelltown

Location : 3. Farrow Rd / Farrow Rd

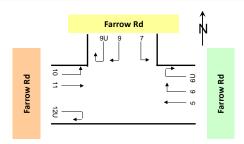
Day/Date : Tue, 25th February 2020

Weather : Fine

Description : Classified Intersection Count

: 15 mins Data

Class 1 Class 2
Classifications Lights Heavies





				Farro	ow Rd					
			irection Through			irection Right Tur			rection ((U Turn)	
		Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
		4	0	4	23	2	25	8	0	8
		9	0	9	38	0	38	2	0	2
		2	0	2	25	3	28	13	0	13
		6	0	6	19	1	20	11	0	11
		2	0	2	22	0	22	11	0	11
		4	0	4	22	3	25	13	1	14
		2	0	2	27	1	28	8	0	8
		7	1	8	13	3	16	6	0	6
		2	0	2	12	2	14	3	0	3
		3	1	4	9	4	13	4	0	4
		1	0	1	17	3	20	1	0	1
		1	0	1	10	3	13	6	0	6
		43	2	45	237	25	262	86	1	87
		0	0	0	33	4	37	8	0	8
		0	0	0	36	3	39	9	0	9
		0	0	0	51	2	53	7	0	7
		0	1	1	38	1	39	2	0	2
		0	0	0	59	2	61	4	0	4
		0	0	0	58	0	58	4	0	4
		0	0	0	101	4	105	9	0	9
		2	0	2	72	1	73	7	0	7
		1	0	1	120	1	121	6	0	6
		0	0	0	67	1	68	4	0	4
		0	0	0	88	2	90	4	0	4
		0	0	0	58	2	60	8	0	8
		3	1	4	781	23	804	72	0	72

Approach				Farro	w Rd											Farro	ow Rd			
Direction		irection Left Tur				irection Right Tur			rection ((U Turn)			irection : Left Turr			irection : Through				ection 1 (U Turn)	
Time Period	Lights	Heavies	Total		Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total		Lights	Heavies	Total
6:30 to 6:45	150	2	152		13	0	13	20	0	20	0	1	1	0	0	0		0	0	0
6:45 to 7:00	109	1	110		21	0	21	69	0	69	0	0	0	0	0	0		0	0	0
7:00 to 7:15	106	4	110		17	0	17	3	0	3	1	0	1	0	0	0		0	0	C
7:15 to 7:30	109	1	110		6	0	6	1	0	1	0	0	0	1	1	2		0	0	C
7:30 to 7:45	79	3	82		6	0	6	0	0	0	3	0	3	0	0	0		0	0	C
7:45 to 8:00	55	5	60		2	0	2	0	0	0	0	0	0	1	0	1		0	0	C
8:00 to 8:15	47	2	49		3	1	4	2	0	2	1	0	1	3	0	3		0	0	(
8:15 to 8:30	36	5	41		1	0	1	1	0	1	0	1	1	0	0	0		0	0	(
8:30 to 8:45	38	1	39		3	1	4	1	0	1	1	0	1	2	0	2		0	0	(
8:45 to 9:00	27	3	30		3	0	3	0	0	0	0	0	0	1	0	1		0	0	(
9:00 to 9:15	23	0	23		1	0	1	0	0	0	0	0	0	1	2	3		0	0	(
9:15 to 9:30	25	2	27		2	0	2	3	0	3	0	0	0	0	0	0		0	0	C
AM Totals	804	29	833		78	2	80	100	0	100	6	2	8	9	3	12		0	0	(
15:30 to 15:45	32	2	34		0	1	1	0	0	0	4	0	4	3	0	3		0	0	C
15:45 to 16:00	32	1	33		1	0	1	1	0	1	2	0	2	0	0	0		0	0	(
l6:00 to 16:15	22	4	26		0	0	0	1	0	1	10	0	10	8	0	8		0	0	(
.6:15 to 16:30	36	4	40		0	0	0	0	0	0	4	0	4	1	0	1		0	0	(
.6:30 to 16:45	17	2	19		0	0	0	0	0	0	5	0	5	3	0	3		0	0	(
.6:45 to 17:00	20	3	23		1	0	1	0	0	0	7	0	7	2	0	2		0	0	(
17:00 to 17:15	19	2	21		0	0	0	1	0	1	5	0	5	2	0	2		0	0	(
.7:15 to 17:30	39	2	41		1	0	1	1	0	1	6	0	6	3	0	3		0	0	
.7:30 to 17:45	27	2	29		1	0	1	2	0	2	7	0	7	2	0	2		0	0	(
.7:45 to 18:00	33	2	35		0	0	0	0	0	0	2	0	2	2	0	2		0	0	C
.8:00 to 18:15	35	2	37		1	0	1	0	0	0	6	0	6	0	0	0		0	0	(
8:15 to 18:30	34	2	36		0	0	0	1	0	1	8	0	8	3	0	3		0	0	(
PM Totals	346	28	374		5	1	6	7	0	7	66	0	66	29	0	29		0	0	(

Client : Varga Traffic Planning

Suburb : Campbelltown

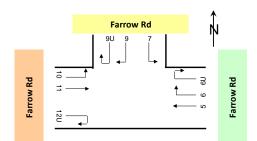
Location : 3. Farrow Rd / Farrow Rd

Day/Date : Tue, 25th February 2020

Weather : Fine

Description : Classified Intersection Count

: Hourly Summary





Approach				Farro	w Rd					
Direction			irection Through			irection Right Tur			rection (U Turn)	
Time Period		Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
6:30 to 7:30		21	0	21	105	6	111	34	0	34
6:45 to 7:45		19	0	19	104	4	108	37	0	37
7:00 to 8:00		14	0	14	88	7	95	48	1	49
7:15 to 8:15		14	0	14	90	5	95	43	1	44
7:30 to 8:30		15	1	16	84	7	91	38	1	39
7:45 to 8:45		15	1	16	74	9	83	30	1	31
8:00 to 9:00		14	2	16	61	10	71	21	0	21
8:15 to 9:15		13	2	15	51	12	63	14	0	14
8:30 to 9:30		7	1	8	48	12	60	14	0	14
AM Totals		43	2	45	237	25	262	86	1	87
15:30 to 16:30		0	1	1	158	10	168	26	0	26
15:45 to 16:45		0	1	1	184	8	192	22	0	22
16:00 to 17:00		0	1	1	206	5	211	17	0	17
16:15 to 17:15		0	1	1	256	7	263	19	0	19
16:30 to 17:30		2	0	2	290	7	297	24	0	24
16:45 to 17:45		3	0	3	351	6	357	26	0	26
17:00 to 18:00		3	0	3	360	7	367	26	0	26
17:15 to 18:15		3	0	3	347	5	352	21	0	21
17:30 to 18:30		1	0	1	333	6	339	22	0	22
PM Totals		3	1	4	781	23	804	72	0	72

Approach				Farro	w Rd											Farro	w Rd			
Direction		irection Left Turr				irection light Tur			rection ((U Turn)			rection 1 eft Turn	-		rection : Through				rection 1 (U Turn)	
Time Period	Lights	Heavies	Total		Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total		Lights	Heavies	,
6:30 to 7:30	474	8	482		57	0	57	93	0	93	1	1	2	1	1	2		0	0	
6:45 to 7:45	403	9	412		50	0	50	73	0	73	4	0	4	1	1	2		0	0	
7:00 to 8:00	349	13	362		31	0	31	4	0	4	4	0	4	2	1	3		0	0	
7:15 to 8:15	290	11	301		17	1	18	3	0	3	4	0	4	5	1	6		0	0	
7:30 to 8:30	217	15	232		12	1	13	3	0	3	4	1	5	4	0	4		0	0	
7:45 to 8:45	176	13	189		9	2	11	4	0	4	2	1	3	6	0	6		0	0	
8:00 to 9:00	148	11	159		10	2	12	4	0	4	2	1	3	6	0	6		0	0	
8:15 to 9:15	124	9	133		8	1	9	2	0	2	1	1	2	4	2	6		0	0	
8:30 to 9:30	113	6	119		9	1	10	4	0	4	1	0	1	4	2	6		0	0	
AM Totals	804	29	833		78	2	80	100	0	100	6	2	8	9	3	12		0	0	
15:30 to 16:30	122	11	133		1	1	2	2	0	2	20	0	20	12	0	12		0	0	
15:45 to 16:45	107	11	118		1	0	1	2	0	2	21	0	21	12	0	12		0	0	
16:00 to 17:00	95	13	108		1	0	1	1	0	1	26	0	26	14	0	14		0	0	
16:15 to 17:15	92	11	103		1	0	1	1	0	1	21	0	21	8	0	8		0	0	
16:30 to 17:30	95	9	104		2	0	2	2	0	2	23	0	23	10	0	10		0	0	
16:45 to 17:45	105	9	114		3	0	3	4	0	4	25	0	25	9	0	9		0	0	
17:00 to 18:00	118	8	126		2	0	2	4	0	4	20	0	20	9	0	9		0	0	
17:15 to 18:15	134	8	142		3	0	3	3	0	3	21	0	21	7	0	7		0	0	
17:30 to 18:30	129	8	137		2	0	2	3	0	3	23	0	23	7	0	7		0	0	
PM Totals	346	28	374		5	1	6	7	0	7	66	0	66	29	0	29		0	0	

Client : Varga Traffic Planning

Suburb : Campbelltown

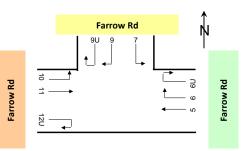
Location : 3. Farrow Rd / Farrow Rd

Day/Date : Tue, 25th February 2020

Weather : Fine

Description : Classified Intersection Count

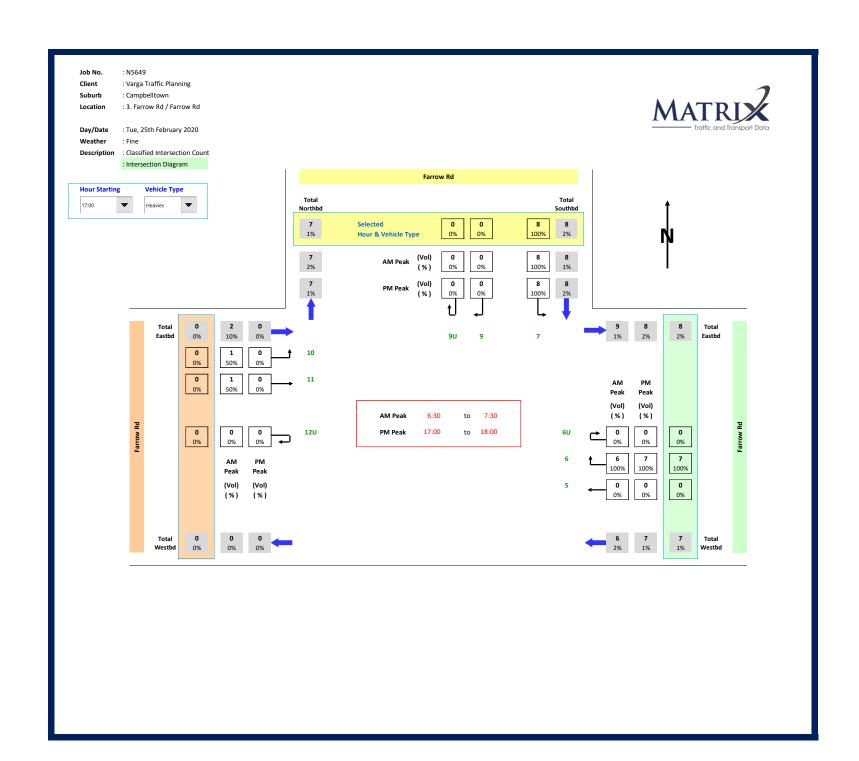
: Peak Hour Summary





	Approach	pach	F	arrow R	d	F	arrow R	d	F	arrow R	d	Total
	Time Period	eriod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand
1	6:30 to 7:30		160	6	166	624	8	632	2	2	4	802
1	17:00 to 18:00		389	7	396	124	8	132	29	0	29	557

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Client : Varga Traffic Planning

Suburb : Campbelltown

Location : 4. Farrow Rd / Badgally Rd / Watsford Rd

Day/Date : Tue, 25th February 2020

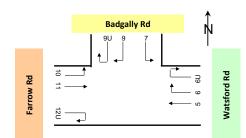
Weather : Fine

Description : Classified Intersection Count

: 15 mins Data

Class 1 Class 2

Classifications Lights Heavies





Approach				Watsf	ord Rd					
Direction			rection Through			irection ight Tur			rection (U Turn	
Time Period		Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	
5:30 to 6:45		4	0	4	0	0	0	0	0	Ī
:45 to 7:00		9	0	9	1	0	1	0	0	Ī
00 to 7:15	***************************************	5	0	5	4	0	4	0	0	Ť
:15 to 7:30		4	0	4	3	1	4	0	0	T
30 to 7:45		5	0	5	6	0	6	1	0	İ
:45 to 8:00		5	0	5	7	0	7	2	0	Ť
:00 to 8:15		9	1	10	5	0	5	1	0	T
:15 to 8:30		9	1	10	2	0	2	1	0	T
30 to 8:45	***************************************	4	1	5	5	1	6	0	0	Ī
15 to 9:00		7	1	8	5	0	5	1	0	T
) to 9:15		8	1	9	5	1	6	1	0	Ī
5 to 9:30		10	0	10	6	0	6	0	0	İ
AM Totals		79	5	84	49	3	52	7	0	
:30 to 15:45		18	0	18	14	2	16	2	0	I
:45 to 16:00		12	0	12	6	0	6	0	0	Ī
5:00 to 16:15		14	0	14	4	0	4	1	0	Ī
5:15 to 16:30		15	1	16	6	1	7	1	0	Ī
5:30 to 16:45		15	0	15	12	1	13	1	0	Ī
:45 to 17:00		9	0	9	4	0	4	2	0	Ī
:00 to 17:15		35	0	35	15	0	15	0	0	Ī
:15 to 17:30		18	0	18	6	0	6	0	0	Ī
30 to 17:45	Accountable of the Control of the Co	19	0	19	14	0	14	0	0	Ī
45 to 18:00	***************************************	6	0	6	3	0	3	0	0	T
0 to 18:15		8	0	8	11	2	13	1	0	T
5 to 18:30	·····	13	0	13	9	0	9	0	0	Ī
M Totals	1	182	1	183	104	6	110	8	0	Ī

Approach				Badga	illy Rd											Farro	ow Rd			
Direction		Direction Left Turi				irection Right Tur			irection ((U Turn)			irection Left Turr			irection : Through				rection 1 (U Turn)	
Time Period	Lights	Heavies	Total		Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total		Lights	Heavies	Total
6:30 to 6:45	6	0	6		41	2	43	2	0	2	28	1	29	4	1	5		10	1	1
6:45 to 7:00	5	1	6		24	1	25	0	0	0	57	1	58	26	0	26		9	0	9
7:00 to 7:15	4	1	5		29	1	30	2	0	2	79	2	81	24	0	24		10	1	1
7:15 to 7:30	9	0	9		22	0	22	2	0	2	75	2	77	35	0	35		10	0	1
7:30 to 7:45	18	0	18		21	2	23	4	0	4	46	3	49	30	0	30		12	1	1
7:45 to 8:00	10	0	10		23	0	23	1	0	1	37	3	40	18	1	19		10	1	1
8:00 to 8:15	3	0	3		16	0	16	0	0	0	29	2	31	18	1	19		8	0	
8:15 to 8:30	7	1	8		10	1	11	0	0	0	26	3	29	16	0	16		8	1	
8:30 to 8:45	7	0	7		8	1	9	1	0	1	17	2	19	16	0	16		4	0	
8:45 to 9:00	2	0	2		4	2	6	1	0	1	12	1	13	20	0	20		5	1	
9:00 to 9:15	4	0	4		8	2	10	1	0	1	12	3	15	9	0	9		0	0	
9:15 to 9:30	9	1	10		3	2	5	0	1	1	14	2	16	4	0	4		2	0	
AM Totals	84	4	88		209	14	223	14	1	15	432	25	457	220	3	223		88	6	9
15:30 to 15:45	5	0	5		11	4	15	1	0	1	21	2	23	7	0	7		4	0	
15:45 to 16:00	4	0	4		8	3	11	2	0	2	37	0	37	12	1	13		4	0	
16:00 to 16:15	8	0	8		12	1	13	2	0	2	25	2	27	10	0	10		13	0	1
16:15 to 16:30	8	0	8		8	1	9	1	0	1	23	4	27	3	1	4		7	1	
16:30 to 16:45	4	1	5		10	1	11	5	0	5	21	2	23	6	0	6		7	0	
16:45 to 17:00	10	0	10		14	0	14	0	0	0	22	2	24	6	0	6		3	0	
17:00 to 17:15	6	0	6		19	3	22	1	0	1	20	2	22	4	0	4		10	1	1
17:15 to 17:30	3	0	3		7	1	8	0	0	0	42	1	43	5	0	5		12	0	:
17:30 to 17:45	10	0	10		13	1	14	1	0	1	52	1	53	2	0	2		9	0	
17:45 to 18:00	5	0	5		20	1	21	1	0	1	17	2	19	8	0	8		7	1	
18:00 to 18:15	3	0	3		10	1	11	0	0	0	40	1	41	8	0	8		13	1	:
.8:15 to 18:30	3	0	3		20	1	21	0	0	0	40	2	42	5	0	5		10	0	:
PM Totals	69	1	70		152	18	170	14	0	14	360	21	381	76	2	78		99	4	1

Client : Varga Traffic Planning

Suburb : Campbelltown

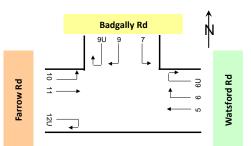
Location : 4. Farrow Rd / Badgally Rd / Watsford Rd

Day/Date : Tue, 25th February 2020

Weather : Fin

Description : Classified Intersection Count

: Hourly Summary





ch				Watsf	ord Rd					
			irection Through			irection Right Tur			rection ((U Turn)	
		Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	
		22	0	22	8	1	9	0	0	
		23	0	23	14	1	15	1	0	
		19	0	19	20	1	21	3	0	_
		23	1	24	21	1	22	4	0	-
		28	2	30	20	0	20	5	0	_
		27	3	30	19	1	20	4	0	Τ
		29	4	33	17	1	18	3	0	J
		28	4	32	17	2	19	3	0	Ī
		29	3	32	21	2	23	2	0	Ī
		79	5	84	49	3	52	7	0	ľ
		59	1	60	30	3	33	4	0	Ī
		56	1	57	28	2	30	3	0	Γ
		53	1	54	26	2	28	5	0	I
		74	1	75	37	2	39	4	0	
		77	0	77	37	1	38	3	0	Ī
		81	0	81	39	0	39	2	0	Ī
		78	0	78	38	0	38	0	0	Ī
		51	0	51	34	2	36	1	0	Ī
	•	46	0	46	37	2	39	1	0	
	ļ	182	1	183	104	6	110	8	0	Ī

Approach				Badga	lly Rd											Farro	w Rd			
Direction		irection Left Turi				irection light Tur			rection 9 (U Turn)			rection : Left Turr			rection Through				ection 1 (U Turn)	
Time Period	Lights	Heavies	Total		Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total		Lights	Heavies	Total
6:30 to 7:30	24	2	26		116	4	120	6	0	6	239	6	245	89	1	90		39	2	41
6:45 to 7:45	36	2	38		96	4	100	8	0	8	257	8	265	115	0	115		41	2	43
7:00 to 8:00	41	1	42		95	3	98	9	0	9	237	10	247	107	1	108		42	3	45
7:15 to 8:15	40	0	40		82	2	84	7	0	7	187	10	197	101	2	103		40	2	42
7:30 to 8:30	38	1	39		70	3	73	5	0	5	138	11	149	82	2	84		38	3	41
7:45 to 8:45	27	1	28		57	2	59	2	0	2	109	10	119	68	2	70		30	2	32
8:00 to 9:00	19	1	20		38	4	42	2	0	2	84	8	92	70	1	71		25	2	27
8:15 to 9:15	20	1	21		30	6	36	3	0	3	67	9	76	61	0	61		17	2	19
8:30 to 9:30	22	1	23		23	7	30	3	1	4	55	8	63	49	0	49		11	1	12
AM Totals	84	4	88		209	14	223	14	1	15	432	25	457	220	3	223		88	6	94
15:30 to 16:30	25	0	25		39	9	48	6	0	6	106	8	114	32	2	34		28	1	29
15:45 to 16:45	24	1	25		38	6	44	10	0	10	106	8	114	31	2	33		31	1	32
16:00 to 17:00	30	1	31		44	3	47	8	0	8	91	10	101	25	1	26		30	1	31
16:15 to 17:15	28	1	29		51	5	56	7	0	7	86	10	96	19	1	20		27	2	29
16:30 to 17:30	23	1	24		50	5	55	6	0	6	105	7	112	21	0	21		32	1	33
16:45 to 17:45	29	0	29		53	5	58	2	0	2	136	6	142	17	0	17		34	1	35
17:00 to 18:00	24	0	24		59	6	65	3	0	3	131	6	137	19	0	19		38	2	40
17:15 to 18:15	21	0	21		50	4	54	2	0	2	151	5	156	23	0	23		41	2	43
17:30 to 18:30	21	0	21		63	4	67	2	0	2	149	6	155	23	0	23		39	2	41
PM Totals	69	1	70		152	18	170	14	0	14	360	21	381	76	2	78		99	4	103

Client : Varga Traffic Planning

Suburb : Campbelltown

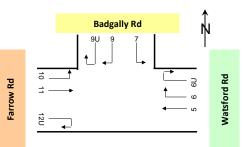
Location : 4. Farrow Rd / Badgally Rd / Watsford Rd

Day/Date : Tue, 25th February 2020

Weather : Fine

Description : Classified Intersection Count

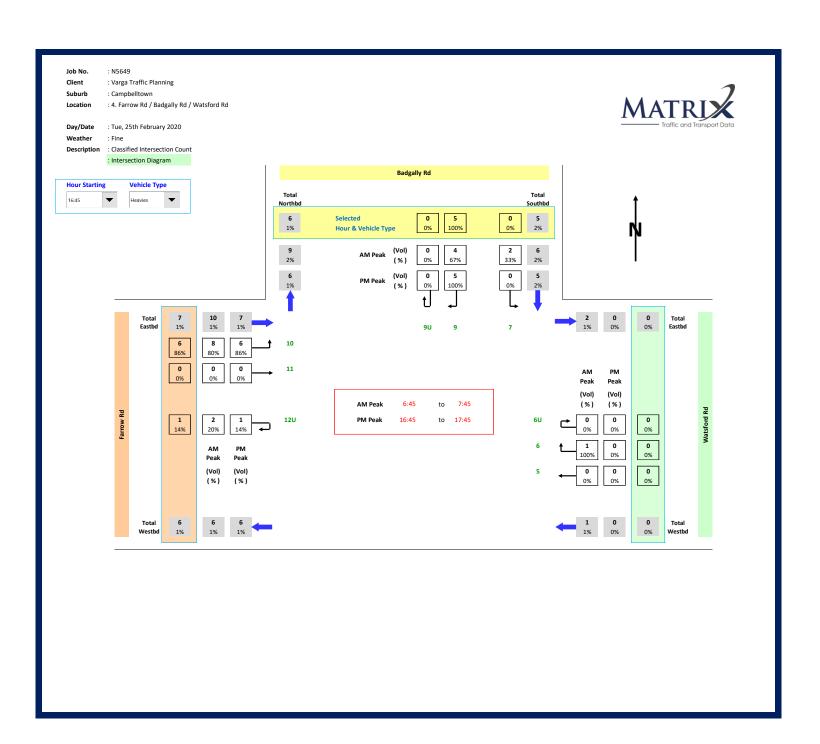
: Peak Hour Summary





	Approach	W	atsford	Rd	Ва	adgally F	Rd	F	arrow R	d	Total
	Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand T
AM	6:45 to 7:45	38	1	39	140	6	146	413	10	423	608
PM	16:45 to 17:45	122	0	122	84	5	89	187	7	194	405

proa	1	ch
ie Pei		riod
	to	7:30
to)	7:45
t	0	8:00
t	0	8:15
1	0	8:30
to		8:45
to)	9:00
to		9:15
to 9	9	:30
/I Totals	als	
to		16:30
to		16:45
to	•	17:00
to		17:15
to	_	17:30
to		17:45
to		18:00
to	•	18:15
to 18:3	18:3	0
/I Totals	als	



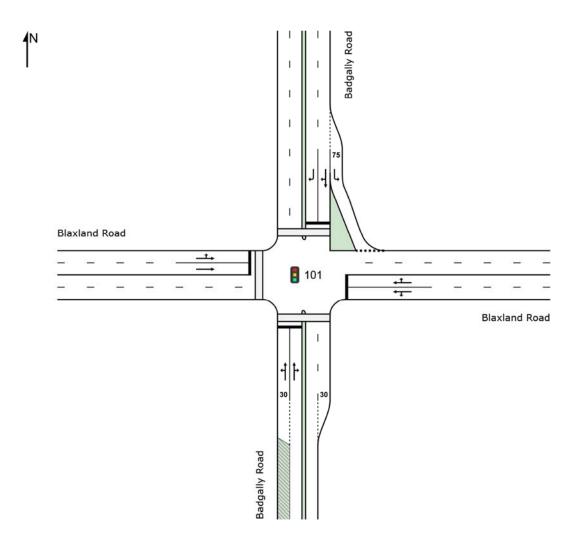
APPENDIX B

SIDRA RESULTS

SITE LAYOUT

Site: 101 [Existing AM]

Blaxland Road & Badgally Road Intersection Site Category: (None) Signals - Fixed Time Isolated



SIDRA INTERSECTION 8.0 | Copyright © 2000-2019 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: VARGA TRAFFIC PLANNING | Created: Tuesday, 3 March, 2020 8:46:21 AM

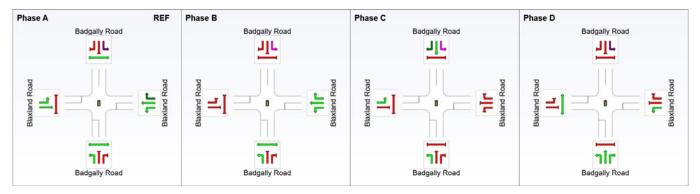
Project: Z:\DATA\Data\Jobs01\Jobs\20work\20067Y_2FarrowRdCampbelltown\SIDRA\200228\BlaxlandRd_BadgallyRd.sip8

INPUT PHASE SEQUENCE

Site: 101 [Existing AM]

Blaxland Road & Badgally Road Intersection Site Category: (None)
Signals - Fixed Time Isolated

Phase Sequence: TCS Plan Reference Phase: Phase A Input Phase Sequence: A, B, C, D Movement Class: All Movement Classes



REF: Reference Phase VAR: Variable Phase



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Organisation: VARGA TRAFFIC PLANNING | Created: Tuesday, 3 March, 2020 8:46:28 AM

Project: Z:\DATA\Data\Jobs01\Jobs\20work\20067Y_2FarrowRdCampbelltown\SIDRA\200228\BlaxlandRd_BadgallyRd.sip8

Site: 101 [Existing AM]

Blaxland Road & Badgally Road Intersection

Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 103 seconds (Site Optimum Cycle Time - Minimum Delay)

Mover	nent Perform	nance - Vehic	cles									
Mov ID	Turn	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Badgally Road											
1	L2	38	2.6	0.284	44.8	LOS D	3.0	22.1	0.94	0.74	0.94	34.7
2	T1	60	13.3	0.284	43.4	LOS D	3.0	22.1	0.95	0.74	0.95	34.1
3	R2	25	0.0	0.284	53.0	LOS D	2.7	19.8	0.96	0.74	0.96	32.4
Approa	ch	123	7.3	0.284	45.8	LOS D	3.0	22.1	0.95	0.74	0.95	33.9
East: B	laxland Road											
4	L2	31	0.0	0.545	26.2	LOS B	16.4	123.1	0.75	0.68	0.75	43.7
5	T1	438	8.7	0.545	20.7	LOS B	16.4	123.1	0.75	0.68	0.75	44.6
6	R2	201	8.5	0.936	81.0	LOS F	13.3	99.9	1.00	1.24	2.08	25.5
Approa	ch	670	8.2	0.936	39.0	LOS C	16.4	123.1	0.83	0.85	1.15	36.4
North: I	Badgally Road											
7	L2	194	9.3	0.200	11.8	LOS A	3.4	26.0	0.44	0.68	0.44	49.4
8	T1	64	6.2	0.538	36.6	LOS C	9.3	68.8	0.92	0.80	0.92	36.1
9	R2	348	6.3	0.538	42.2	LOS C	9.3	68.8	0.92	0.81	0.92	35.0
Approa	ch	606	7.3	0.538	31.9	LOS C	9.3	68.8	0.76	0.77	0.76	38.8
West: E	Blaxland Road											
10	L2	230	5.7	0.752	35.1	LOS C	21.2	156.0	0.94	0.88	1.10	38.5
11	T1	768	5.9	0.752	31.7	LOS C	22.4	164.8	0.94	0.86	1.02	39.1
Approa	ch	998	5.8	0.752	32.4	LOS C	22.4	164.8	0.94	0.87	1.03	39.0
All Veh	icles	2397	6.9	0.936	34.8	LOSC	22.4	164.8	0.87	0.83	0.99	37.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov		Demand	Average	Level of	Average Back o		Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		
P1	South Full Crossing	50	45.8	LOS E	0.1	0.1	0.94	0.94
P3	North Full Crossing	50	45.8	LOS E	0.1	0.1	0.94	0.94
P4	West Full Crossing	50	45.8	LOS E	0.1	0.1	0.94	0.94
All Ped	estrians	150	45.8	LOS E			0.94	0.94

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Existing PM]

Blaxland Road & Badgally Road Intersection

Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 101 seconds (Site Optimum Cycle Time - Minimum Delay)

Moven	nent Perform	ance - Vehi	cles									
Mov ID	Turn	Deman Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: I	Badgally Road											
1	L2	78	1.3	0.580	45.5	LOS D	6.4	45.8	0.98	0.79	0.98	34.6
2	T1	142	3.5	0.580	44.5	LOS D	6.4	45.8	0.99	0.79	0.99	33.9
3	R2	40	7.5	0.580	54.0	LOS D	5.8	42.0	1.00	0.79	1.01	32.3
Approa	ch	260	3.5	0.580	46.3	LOS D	6.4	45.8	0.99	0.79	0.99	33.8
East: B	laxland Road											
4	L2	31	3.2	0.814	30.7	LOS C	33.4	237.5	0.91	0.86	0.94	41.5
5	T1	840	1.8	0.814	27.5	LOS B	33.4	237.5	0.92	0.88	1.03	41.1
6	R2	287	3.5	0.814	51.4	LOS D	17.3	124.2	1.00	1.09	1.68	32.6
Approa	ch	1158	2.2	0.814	33.5	LOS C	33.4	237.5	0.94	0.93	1.19	38.6
North: E	Badgally Road											
7	L2	225	3.1	0.204	10.7	LOS A	3.6	25.7	0.42	0.67	0.42	50.3
8	T1	45	11.1	0.550	38.8	LOS C	8.4	60.8	0.94	0.81	0.94	35.1
9	R2	328	1.2	0.550	44.3	LOS D	8.4	60.8	0.94	0.81	0.94	34.3
Approa	ch	598	2.7	0.550	31.3	LOS C	8.4	60.8	0.74	0.76	0.74	39.1
West: E	Blaxland Road											
10	L2	357	1.4	0.896	48.7	LOS D	22.4	159.3	1.00	1.07	1.47	33.2
11	T1	507	3.0	0.896	51.9	LOS D	22.7	162.7	1.00	1.09	1.38	32.2
Approa	ch	864	2.3	0.896	50.6	LOS D	22.7	162.7	1.00	1.08	1.42	32.6
All Vehi	cles	2880	2.5	0.896	39.3	LOSC	33.4	237.5	0.92	0.93	1.15	36.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mov	ment Performance - Pedestrians	Demand	Average	Level of	Average Back o	f Queue	Prop.	Effective
ID	Description	Flow ped/h	Delay sec	Service	Pedestrian ped	Distance m	Queued	Stop Rate
P1	South Full Crossing	50	44.8	LOS E	0.1	0.1	0.94	0.94
P3	North Full Crossing	50	44.8	LOS E	0.1	0.1	0.94	0.94
P4	West Full Crossing	50	44.8	LOS E	0.1	0.1	0.94	0.94
All Ped	lestrians	150	44.8	LOS E			0.94	0.94

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Proposed AM]

Blaxland Road & Badgally Road Intersection

Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 103 seconds (Site Optimum Cycle Time - Minimum Delay)

Moven	nent Perforn	nance - Vehic	cles									
Mov ID	Turn	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: I	Badgally Road	l										
1	L2	72	1.4	0.316	42.7	LOS D	4.1	29.9	0.92	0.75	0.92	35.1
2	T1	60	13.3	0.316	43.5	LOS D	4.1	29.9	0.95	0.75	0.95	34.0
3	R2	25	0.0	0.316	53.2	LOS D	3.0	22.2	0.96	0.74	0.96	32.4
Approa	ch	157	5.7	0.316	44.7	LOS D	4.1	29.9	0.94	0.75	0.94	34.2
East: B	laxland Road											
4	L2	31	0.0	0.545	26.2	LOS B	16.4	123.1	0.75	0.68	0.75	43.7
5	T1	438	8.7	0.545	20.7	LOS B	16.4	123.1	0.75	0.68	0.75	44.6
6	R2	201	8.5	0.936	81.0	LOS F	13.3	99.9	1.00	1.24	2.08	25.5
Approa	ch	670	8.2	0.936	39.0	LOS C	16.4	123.1	0.83	0.85	1.15	36.4
North: E	Badgally Road											
7	L2	194	9.3	0.200	11.8	LOS A	3.4	26.0	0.44	0.68	0.44	49.4
8	T1	64	6.2	0.538	36.6	LOS C	9.3	68.8	0.92	0.80	0.92	36.1
9	R2	348	6.3	0.538	42.2	LOS C	9.3	68.8	0.92	0.81	0.92	35.0
Approa	ch	606	7.3	0.538	31.9	LOS C	9.3	68.8	0.76	0.77	0.76	38.8
West: E	laxland Road											
10	L2	230	5.7	0.752	35.1	LOS C	21.2	156.0	0.94	0.88	1.10	38.5
11	T1	768	5.9	0.752	31.7	LOS C	22.4	164.8	0.94	0.86	1.02	39.1
Approa	ch	998	5.8	0.752	32.4	LOS C	22.4	164.8	0.94	0.87	1.03	39.0
All Vehi	cles	2431	6.8	0.936	34.9	LOSC	22.4	164.8	0.87	0.83	0.99	37.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov		Demand	Average	Level of	Average Back o		Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		
P1	South Full Crossing	50	45.8	LOS E	0.1	0.1	0.94	0.94
P3	North Full Crossing	50	45.8	LOS E	0.1	0.1	0.94	0.94
P4	West Full Crossing	50	45.8	LOS E	0.1	0.1	0.94	0.94
All Ped	estrians	150	45.8	LOS E			0.94	0.94

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [Proposed PM]

Blaxland Road & Badgally Road Intersection

Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 101 seconds (Site Optimum Cycle Time - Minimum Delay)

Moven	nent Perform	ance - Vehic	cles									
Mov ID	Turn	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South: I	Badgally Road											
1	L2	91	1.1	0.596	45.8	LOS D	6.9	49.1	0.99	0.79	0.99	34.4
2	T1	142	3.5	0.596	44.9	LOS D	6.9	49.1	0.99	0.80	1.01	33.8
3	R2	40	7.5	0.596	54.2	LOS D	5.9	43.3	1.00	0.80	1.02	32.3
Approa	ch	273	3.3	0.596	46.6	LOS D	6.9	49.1	0.99	0.80	1.00	33.7
East: B	laxland Road											
4	L2	31	3.2	0.814	30.7	LOS C	33.4	237.5	0.91	0.86	0.94	41.5
5	T1	840	1.8	0.814	27.5	LOS B	33.4	237.5	0.92	0.88	1.03	41.1
6	R2	287	3.5	0.814	51.4	LOS D	17.3	124.2	1.00	1.09	1.68	32.6
Approa	ch	1158	2.2	0.814	33.5	LOS C	33.4	237.5	0.94	0.93	1.19	38.6
North: E	Badgally Road											
7	L2	225	3.1	0.204	10.7	LOS A	3.6	25.7	0.42	0.67	0.42	50.3
8	T1	45	11.1	0.550	38.8	LOS C	8.4	60.8	0.94	0.81	0.94	35.1
9	R2	328	1.2	0.550	44.3	LOS D	8.4	60.8	0.94	0.81	0.94	34.3
Approa	ch	598	2.7	0.550	31.3	LOS C	8.4	60.8	0.74	0.76	0.74	39.1
West: E	laxland Road											
10	L2	357	1.4	0.896	48.7	LOS D	22.4	159.3	1.00	1.07	1.47	33.2
11	T1	507	3.0	0.896	51.9	LOS D	22.7	162.7	1.00	1.09	1.38	32.2
Approa	ch	864	2.3	0.896	50.6	LOS D	22.7	162.7	1.00	1.08	1.42	32.6
All Vehi	cles	2893	2.5	0.896	39.4	LOSC	33.4	237.5	0.92	0.93	1.15	36.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mov	ment Performance - Pedestrians	Demand	Average	Level of	Average Back o	f Queue	Prop.	Effective
ID	Description	Flow ped/h	Delay sec	Service	Pedestrian ped	Distance m	Queued	Stop Rate
P1	South Full Crossing	50	44.8	LOS E	0.1	0.1	0.94	0.94
P3	North Full Crossing	50	44.8	LOS E	0.1	0.1	0.94	0.94
P4	West Full Crossing	50	44.8	LOS E	0.1	0.1	0.94	0.94
All Ped	lestrians	150	44.8	LOS E			0.94	0.94

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

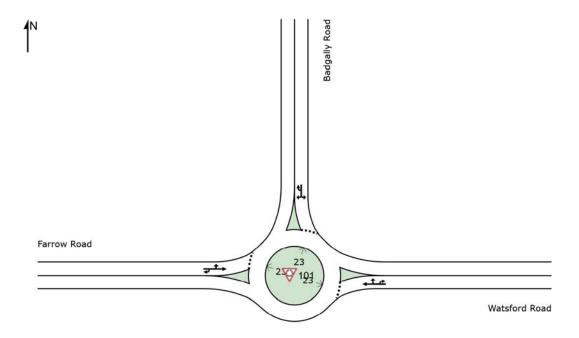
Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SITE LAYOUT

Site: 101 [Existing AM]

Farrow Road, Watsford Road & Badgally Road Roundabout Site Category: (None)
Roundabout



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Organisation: VARGA TRAFFIC PLANNING | Created: Friday, 28 February, 2020 3:21:39 PM
Project: Z:\DATA\Data\Jobs01\Jobs\20work\20067Y_2FarrowRdCampbelltown\SIDRA\200228\BadgallyRd_FarrowRd.sip8

Site: 101 [Existing AM]

Farrow Road, Watsford Road & Badgally Road Roundabout Site Category: (None) Roundabout

Mov	Turn	Demand	d Flows	Deg.	Average	Level of	95% Back c	of Queue	Prop.	Effective	Aver. No.	Average
ID		Total veh/h	HV %	Satn v/c	Delay	Service	Vehicles veh	Distance m	Queued	Stop Rate	Cycles	Speed km/h
East: W	atsford Road											
5	T1	23	0.0	0.033	4.5	LOSA	0.2	1.1	0.31	0.52	0.31	54.4
6	R2	16	6.3	0.033	9.6	LOSA	0.2	1.1	0.31	0.52	0.31	54.2
6u	U	1	0.0	0.033	11.7	LOS A	0.2	1.1	0.31	0.52	0.31	55.5
Approa	ch	40	2.5	0.033	6.7	LOSA	0.2	1.1	0.31	0.52	0.31	54.4
North: E	Badgally Road											
7	L2	40	5.0	0.126	4.6	LOSA	0.6	4.7	0.33	0.60	0.33	51.8
9	R2	104	3.8	0.126	9.7	LOSA	0.6	4.7	0.33	0.60	0.33	53.1
9u	U	8	0.0	0.126	11.7	LOS A	0.6	4.7	0.33	0.60	0.33	54.3
Approa	ch	152	3.9	0.126	8.4	LOSA	0.6	4.7	0.33	0.60	0.33	52.9
West: F	arrow Road											
10	L2	273	2.9	0.277	3.9	LOSA	1.7	12.3	0.13	0.46	0.13	54.7
11	T1	115	0.0	0.277	4.0	LOSA	1.7	12.3	0.13	0.46	0.13	56.3
12u	U	45	4.4	0.277	11.1	LOS A	1.7	12.3	0.13	0.46	0.13	57.3
Approa	ch	433	2.3	0.277	4.6	LOSA	1.7	12.3	0.13	0.46	0.13	55.4
All Vehi	cles	625	2.7	0.277	5.7	LOSA	1.7	12.3	0.19	0.50	0.19	54.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: VARGA TRAFFIC PLANNING | Processed: Friday, 28 February, 2020 3:16:28 PM
Project: Z:\DATA\Data\Jobs01\Jobs\20work\20067Y_2FarrowRdCampbelltown\SIDRA\200228\BadgallyRd_FarrowRd.sip8

Site: 101 [Existing PM]

Farrow Road, Watsford Road & Badgally Road Roundabout Site Category: (None) Roundabout

	ent Perform											
Mov ID	Turn	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: W	atsford Road	VOII/II		V/0	555		7011					1(11)1
5	T1	23	0.0	0.029	4.2	LOSA	0.1	1.0	0.23	0.50	0.23	54.8
6	R2	14	0.0	0.029	9.2	LOSA	0.1	1.0	0.23	0.50	0.23	54.8
6u	U	1	0.0	0.029	11.4	LOS A	0.1	1.0	0.23	0.50	0.23	55.9
Approac	ch	38	0.0	0.029	6.3	LOSA	0.1	1.0	0.23	0.50	0.23	54.8
North: E	Badgally Road											
7	L2	29	0.0	0.067	3.9	LOSA	0.3	2.3	0.17	0.57	0.17	52.8
9	R2	58	8.6	0.067	9.2	LOSA	0.3	2.3	0.17	0.57	0.17	53.8
9u	U	2	0.0	0.067	11.2	LOS A	0.3	2.3	0.17	0.57	0.17	55.2
Approac	ch	89	5.6	0.067	7.5	LOSA	0.3	2.3	0.17	0.57	0.17	53.5
West: F	arrow Road											
10	L2	142	4.2	0.126	3.8	LOSA	0.7	4.8	0.09	0.50	0.09	54.4
11	T1	17	0.0	0.126	3.9	LOSA	0.7	4.8	0.09	0.50	0.09	56.0
12u	U	35	2.9	0.126	11.1	LOS A	0.7	4.8	0.09	0.50	0.09	57.1
Approac	ch	194	3.6	0.126	5.1	LOSA	0.7	4.8	0.09	0.50	0.09	55.0
All Vehic	cles	321	3.7	0.126	5.9	LOSA	0.7	4.8	0.13	0.52	0.13	54.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: VARGA TRAFFIC PLANNING | Processed: Friday, 28 February, 2020 3:17:47 PM
Project: Z:\DATA\Data\Jobs01\Jobs\20work\20067Y_2FarrowRdCampbelltown\SIDRA\200228\BadgallyRd_FarrowRd.sip8

Site: 101 [Proposed AM]

Farrow Road, Watsford Road & Badgally Road Roundabout Site Category: (None) Roundabout

Mover	nent Perfori	nance - Veh	icles									
Mov ID	Turn	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: V	Vatsford Road											
5	T1	121	0.0	0.113	4.6	LOSA	0.6	4.3	0.34	0.47	0.34	55.2
6	R2	16	6.3	0.113	9.7	LOSA	0.6	4.3	0.34	0.47	0.34	55.0
6u	U	1	0.0	0.113	11.7	LOSA	0.6	4.3	0.34	0.47	0.34	56.4
Approa	ıch	138	0.7	0.113	5.2	LOS A	0.6	4.3	0.34	0.47	0.34	55.2
North: I	Badgally Road	t										
7	L2	40	5.0	0.146	5.6	LOSA	0.8	5.7	0.50	0.67	0.50	51.4
9	R2	104	3.8	0.146	10.7	LOS A	0.8	5.7	0.50	0.67	0.50	52.6
9u	U	8	0.0	0.146	12.8	LOS A	0.8	5.7	0.50	0.67	0.50	53.8
Approa	ıch	152	3.9	0.146	9.5	LOSA	0.8	5.7	0.50	0.67	0.50	52.4
West: F	arrow Road											
10	L2	273	2.9	0.396	3.9	LOS A	2.9	20.3	0.15	0.43	0.15	54.8
11	T1	315	0.0	0.396	4.0	LOS A	2.9	20.3	0.15	0.43	0.15	56.3
12u	U	45	4.4	0.396	11.2	LOS A	2.9	20.3	0.15	0.43	0.15	57.4
Approa	ıch	633	1.6	0.396	4.4	LOSA	2.9	20.3	0.15	0.43	0.15	55.7
All Veh	icles	923	1.8	0.396	5.4	LOSA	2.9	20.3	0.24	0.48	0.24	55.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: VARGA TRAFFIC PLANNING | Processed: Wednesday, 4 March, 2020 2:03:38 PM
Project: Z:\DATA\Data\Jobs\01\Jobs\02\work\02067Y_2FarrowRdCampbelltown\SIDRA\0200304\BadgallyRd_FarrowRd_WatsfordRd.sip8

Site: 101 [Proposed PM]

Farrow Road, Watsford Road & Badgally Road Roundabout Site Category: (None) Roundabout

Mover	nent Perforr	nance - Veh	icles									
Mov ID	Turn	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: W	atsford Road											
5	T1	180	0.0	0.146	4.3	LOSA	0.8	5.4	0.26	0.43	0.26	55.8
6	R2	14	0.0	0.146	9.3	LOSA	0.8	5.4	0.26	0.43	0.26	55.8
6u	U	1	0.0	0.146	11.4	LOS A	0.8	5.4	0.26	0.43	0.26	57.0
Approa	ch	195	0.0	0.146	4.7	LOS A	0.8	5.4	0.26	0.43	0.26	55.8
North: I	Badgally Road	t										
7	L2	29	0.0	0.073	4.3	LOS A	0.3	2.6	0.28	0.58	0.28	52.5
9	R2	58	8.6	0.073	9.5	LOS A	0.3	2.6	0.28	0.58	0.28	53.5
9u	U	2	0.0	0.073	11.5	LOS A	0.3	2.6	0.28	0.58	0.28	54.9
Approa	ch	89	5.6	0.073	7.9	LOSA	0.3	2.6	0.28	0.58	0.28	53.2
West: F	arrow Road											
10	L2	142	4.2	0.172	3.8	LOS A	1.0	6.8	0.09	0.47	0.09	54.7
11	T1	92	0.0	0.172	3.9	LOS A	1.0	6.8	0.09	0.47	0.09	56.2
12u	U	35	2.9	0.172	11.1	LOS A	1.0	6.8	0.09	0.47	0.09	57.3
Approa	ch	269	2.6	0.172	4.8	LOSA	1.0	6.8	0.09	0.47	0.09	55.5
All Veh	cles	553	2.2	0.172	5.3	LOSA	1.0	6.8	0.18	0.47	0.18	55.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

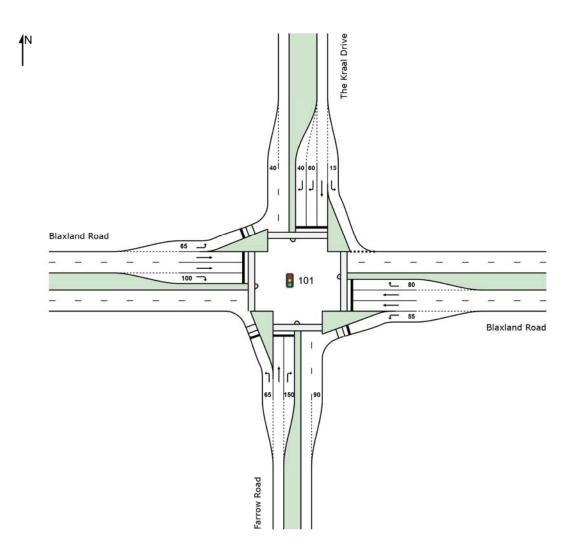
HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: Z:\DATA\Data\Jobs\01\Jobs\02\work\02067Y_2FarrowRdCampbelltown\SIDRA\0200304\BadgallyRd_FarrowRd_WatsfordRd.sip8

SITE LAYOUT

Site: 101 [Existing AM]

Blaxland Road, Farrow Road & The Kraal Drive Intersection Site Category: (None)
Signals - Fixed Time Isolated



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Project: Z:\DATA\Data\Jobs01\Jobs\20work\20067Y_2FarrowRdCampbelltown\SIDRA\200228\BlaxlandRd_FarrowRd_TheKraalDr.sip8

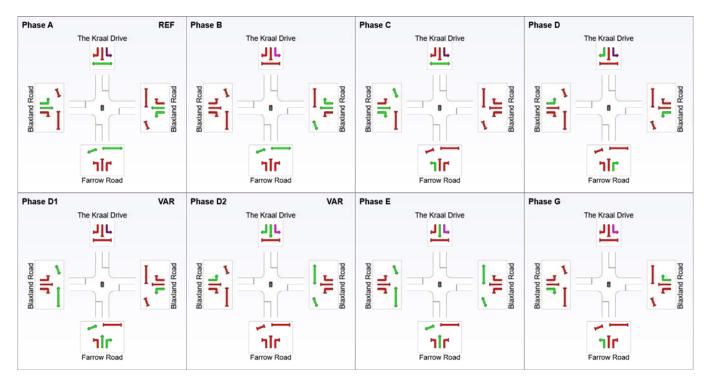
INPUT PHASE SEQUENCE

Site: 101 [Existing AM]

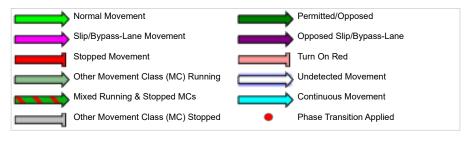
Blaxland Road, Farrow Road & The Kraal Drive Intersection Site Category: (None)
Signals - Fixed Time Isolated

Phase Sequence: TCS Reference Phase: Phase A

Input Phase Sequence: A, B, C, D, D1*, D2*, E, G
Movement Class: All Movement Classes



REF: Reference Phase VAR: Variable Phase



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Site: 101 [Existing AM]

Blaxland Road, Farrow Road & The Kraal Drive Intersection

Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 125 seconds (Site Optimum Cycle Time - Minimum Delay)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Mover	nent Perforn	nance - Vehi	cles									
Mov	Turn		d Flows	Deg.	Average	Level of	95% Back o		Prop.	Effective	Aver. No.	Average
ID		Total veh/h	HV %	Satn v/c	Delay	Service	Vehicles veh	Distance	Queued	Stop Rate	Cycles	Speed km/h
South:	Farrow Road	ven/n	70	V/C	sec		ven	m				KIII/II
1	L2	60	10.0	0.288	34.6	LOS C	1.8	13.9	0.95	0.74	0.95	37.9
2	T1	27	11.1	0.116	52.7	LOS D	1.5	11.3	0.92	0.67	0.92	32.3
3	R2	5	40.0	0.048	66.5	LOS E	0.3	2.8	0.96	0.65	0.96	28.2
Approa	ch	92	12.0	0.288	41.7	LOS C	1.8	13.9	0.94	0.72	0.94	35.4
East: B	laxland Road											
4	L2	21	33.3	0.042	21.8	LOS B	0.5	4.7	0.70	0.67	0.70	43.2
5	T1	693	5.9	0.518	33.6	LOS C	16.7	122.7	0.84	0.73	0.84	38.8
6	R2	30	10.0	0.180	37.0	LOS C	1.1	8.3	0.95	0.71	0.95	36.9
Approa	ch	744	6.9	0.518	33.4	LOS C	16.7	122.7	0.84	0.72	0.84	38.8
North:	The Kraal Driv	е										
7	L2	70	1.4	0.064	12.2	LOSA	1.3	9.6	0.39	0.64	0.39	49.4
8	T1	29	10.3	0.076	42.7	LOS D	1.4	10.8	0.84	0.62	0.84	35.4
9	R2	401	2.0	0.843	68.8	LOS E	13.0	92.8	1.00	0.95	1.27	28.0
Approa	ch	500	2.4	0.843	59.4	LOS E	13.0	92.8	0.90	0.88	1.12	30.2
West: E	Blaxland Road											
10	L2	453	2.4	0.597	21.3	LOS B	11.9	84.7	0.83	0.82	0.83	44.0
11	T1	976	5.8	0.836	32.6	LOS C	25.7	189.3	0.96	0.91	1.07	39.2
12	R2	158	5.1	0.735	38.4	LOS C	5.4	39.4	1.00	0.84	1.11	36.2
Approa	ch	1587	4.8	0.836	30.0	LOSC	25.7	189.3	0.93	0.88	1.00	40.1
All Veh	icles	2923	5.1	0.843	36.2	LOSC	25.7	189.3	0.90	0.83	0.98	37.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Moven	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	f Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	50	56.8	LOS E	0.2	0.2	0.95	0.95
P1B	South Slip/Bypass Lane Crossing	50	25.6	LOS C	0.1	0.1	0.91	0.91
P2	East Full Crossing	50	56.8	LOS E	0.2	0.2	0.95	0.95
P2B	East Slip/Bypass Lane Crossing	50	25.6	LOS C	0.1	0.1	0.91	0.91
P3	North Full Crossing	50	35.7	LOS D	0.1	0.1	0.91	0.91
P4	West Full Crossing	50	56.8	LOS E	0.2	0.2	0.95	0.95
P4B	West Slip/Bypass Lane Crossing	50	27.5	LOS C	0.1	0.1	0.91	0.91
All Ped	estrians	350	40.7	LOSE			0.93	0.93

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: Z:\DATA\Data\Jobs01\Jobs\20work\20067Y_2FarrowRdCampbelltown\SIDRA\200228\BlaxlandRd_FarrowRd_TheKraalDr.sip8

Site: 101 [Existing PM]

Blaxland Road, Farrow Road & The Kraal Drive Intersection

Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 145 seconds (Site Optimum Cycle Time - Minimum Delay)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Mover	nent Perforr	nance - Vehic	cles									
Mov ID	Turn	Demano Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	f Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Farrow Road											
1	L2	347	1.7	0.914	55.2	LOS D	17.9	127.0	1.00	1.00	1.32	31.3
2	T1	58	3.4	0.245	62.6	LOS E	3.8	27.1	0.94	0.72	0.94	29.7
3	R2	10	20.0	0.081	74.8	LOS F	0.7	5.6	0.96	0.68	0.96	26.6
Approa	ch	415	2.4	0.914	56.7	LOS E	17.9	127.0	0.99	0.95	1.26	31.0
East: B	laxland Road											
4	L2	12	8.3	0.020	23.3	LOS B	0.3	2.4	0.69	0.65	0.69	42.9
5	T1	1112	1.3	0.885	57.6	LOS E	43.1	304.6	0.99	0.99	1.13	31.0
6	R2	105	2.9	0.418	39.3	LOS C	4.2	30.2	0.96	0.77	0.96	36.1
Approa	ch	1229	1.5	0.885	55.7	LOS D	43.1	304.6	0.98	0.97	1.11	31.4
North:	The Kraal Driv	'e										
7	L2	85	1.2	0.072	8.6	LOSA	1.2	8.3	0.26	0.62	0.26	51.9
8	T1	27	0.0	0.084	54.9	LOS D	1.6	11.3	0.88	0.64	0.88	31.7
9	R2	337	1.2	0.902	89.0	LOS F	13.5	95.7	1.00	1.01	1.44	24.3
Approa	ch	449	1.1	0.902	71.7	LOS F	13.5	95.7	0.85	0.91	1.18	27.4
West: E	Blaxland Road											
10	L2	421	1.0	0.602	25.6	LOS B	13.3	94.1	0.86	0.82	0.86	41.9
11	T1	717	2.4	0.501	25.8	LOS B	15.7	112.0	0.82	0.71	0.82	42.2
12	R2	98	8.2	0.270	32.6	LOS C	3.2	24.2	0.88	0.76	0.88	38.4
Approa	ch	1236	2.3	0.602	26.3	LOS B	15.7	112.0	0.84	0.75	0.84	41.8
All Veh	icles	3329	1.9	0.914	47.1	LOS D	43.1	304.6	0.91	0.88	1.04	33.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Moven	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	f Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	50	66.8	LOS F	0.2	0.2	0.96	0.96
P1B	South Slip/Bypass Lane Crossing	50	30.6	LOS D	0.1	0.1	0.92	0.92
P2	East Full Crossing	50	66.8	LOS F	0.2	0.2	0.96	0.96
P2B	East Slip/Bypass Lane Crossing	50	30.6	LOS D	0.1	0.1	0.92	0.92
P3	North Full Crossing	50	40.8	LOS E	0.1	0.1	0.92	0.92
P4	West Full Crossing	50	66.8	LOS F	0.2	0.2	0.96	0.96
P4B	West Slip/Bypass Lane Crossing	50	35.1	LOS D	0.1	0.1	0.92	0.92
All Ped	estrians	350	48.2	LOS E			0.94	0.94

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: Z:\DATA\Data\Jobs01\Jobs\20work\20067Y_2FarrowRdCampbelltown\SIDRA\200228\BlaxlandRd_FarrowRd_TheKraalDr.sip8

Site: 101 [Proposed AM]

Blaxland Road, Farrow Road & The Kraal Drive Intersection

Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 125 seconds (Site Optimum Cycle Time - Minimum Delay)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Mover	nent Perforn	nance - Vehi	cles									
Mov	Turn		d Flows	Deg.	Average	Level of	95% Back o		Prop.	Effective	Aver. No.	Average
ID		Total	HV %	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
South:	Farrow Road	veh/h	%	v/c	sec		veh	m				km/h
1	L2	94	6.4	0.413	34.6	LOS C	2.9	21.2	0.96	0.77	0.96	38.0
2	T1	27	11.1	0.116	52.7	LOS D	1.5	11.3	0.92	0.67	0.92	32.3
3	R2	5	40.0	0.048	66.5	LOSE	0.3	2.8	0.96	0.65	0.96	28.2
Approa		126	8.7	0.413	39.7	LOS C	2.9	21.2	0.95	0.74	0.95	36.1
		120	0.7	0.413	39.1	L03 C	2.5	21.2	0.33	0.74	0.93	30.1
East: B	laxland Road											
4	L2	21	33.3	0.043	22.3	LOS B	0.5	4.7	0.71	0.67	0.71	42.9
5	T1	727	5.6	0.555	34.8	LOS C	18.0	131.8	0.86	0.75	0.86	38.3
6	R2	30	10.0	0.166	36.4	LOS C	1.1	8.2	0.94	0.71	0.94	37.1
Approa	ch	778	6.6	0.555	34.6	LOS C	18.0	131.8	0.86	0.74	0.86	38.3
North:	The Kraal Driv	е										
7	L2	70	1.4	0.064	12.6	LOSA	1.4	9.8	0.39	0.64	0.39	49.2
8	T1	29	10.3	0.076	42.7	LOS D	1.4	10.8	0.84	0.62	0.84	35.4
9	R2	401	2.0	0.843	68.8	LOS E	13.0	92.8	1.00	0.95	1.27	28.0
Approa	ch	500	2.4	0.843	59.4	LOS E	13.0	92.8	0.91	0.88	1.12	30.2
West: E	Blaxland Road											
10	L2	453	2.4	0.608	21.8	LOS B	12.2	86.9	0.84	0.82	0.84	43.8
11	T1	976	5.8	0.859	36.1	LOS C	27.2	200.2	0.97	0.95	1.12	37.8
12	R2	191	4.2	0.827	41.9	LOS C	7.0	50.9	1.00	0.91	1.24	35.0
Approa	ch	1620	4.7	0.859	32.8	LOS C	27.2	200.2	0.94	0.91	1.06	38.9
All Veh	icles	3024	5.0	0.859	37.9	LOSC	27.2	200.2	0.91	0.85	1.01	36.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Moven	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	f Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	50	56.8	LOS E	0.2	0.2	0.95	0.95
P1B	South Slip/Bypass Lane Crossing	50	25.6	LOS C	0.1	0.1	0.91	0.91
P2	East Full Crossing	50	56.8	LOS E	0.2	0.2	0.95	0.95
P2B	East Slip/Bypass Lane Crossing	50	25.6	LOS C	0.1	0.1	0.91	0.91
P3	North Full Crossing	50	35.7	LOS D	0.1	0.1	0.91	0.91
P4	West Full Crossing	50	56.8	LOS E	0.2	0.2	0.95	0.95
P4B	West Slip/Bypass Lane Crossing	50	27.5	LOS C	0.1	0.1	0.91	0.91
All Ped	estrians	350	40.7	LOSE			0.93	0.93

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [Proposed PM]

Blaxland Road, Farrow Road & The Kraal Drive Intersection

Site Category: (None)
Signals - Fixed Time Isolated Cycle Time = 140 seconds (Site Optimum Cycle Time - Minimum Delay)

Variable Sequence Analysis applied. The results are given for the selected output sequence.

Mover	nent Perforr	nance - Vehic	cles									
Mov ID	Turn	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	f Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:	Farrow Road											
1	L2	360	1.7	0.915	54.1	LOS D	18.1	128.2	1.00	1.00	1.33	31.6
2	T1	58	3.4	0.250	61.0	LOS E	3.6	26.3	0.95	0.72	0.95	30.1
3	R2	10	20.0	0.054	66.0	LOS E	0.6	5.1	0.92	0.68	0.92	28.5
Approa	ch	428	2.3	0.915	55.3	LOS D	18.1	128.2	0.99	0.96	1.27	31.3
East: B	laxland Road											
4	L2	12	8.3	0.019	21.1	LOS B	0.3	2.1	0.66	0.65	0.66	44.0
5	T1	1125	1.2	0.917	65.2	LOS E	45.8	324.3	1.00	1.07	1.23	29.1
6	R2	105	2.9	0.404	37.9	LOS C	4.1	29.4	0.95	0.77	0.95	36.6
Approa	ch	1242	1.4	0.917	62.5	LOS E	45.8	324.3	0.99	1.04	1.20	29.7
North:	The Kraal Driv	re										
7	L2	85	1.2	0.073	8.7	LOSA	1.2	8.3	0.26	0.62	0.26	51.8
8	T1	27	0.0	0.114	59.5	LOS E	1.7	11.6	0.92	0.67	0.92	30.5
9	R2	337	1.2	0.901	86.6	LOS F	13.1	92.8	1.00	1.01	1.45	24.7
Approa	ch	449	1.1	0.901	70.2	LOS E	13.1	92.8	0.86	0.92	1.19	27.8
West: E	Blaxland Road											
10	L2	421	1.0	0.627	25.9	LOS B	13.1	92.5	0.88	0.83	0.88	41.8
11	T1	717	2.4	0.512	25.5	LOS B	15.3	109.3	0.83	0.72	0.83	42.4
12	R2	150	5.3	0.391	32.0	LOSC	4.9	35.6	0.90	0.78	0.90	38.7
Approa	ch	1288	2.3	0.627	26.4	LOS B	15.3	109.3	0.86	0.76	0.86	41.7
All Veh	icles	3407	1.8	0.917	48.9	LOS D	45.8	324.3	0.92	0.91	1.08	33.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Moven	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	f Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	50	64.3	LOS F	0.2	0.2	0.96	0.96
P1B	South Slip/Bypass Lane Crossing	50	29.4	LOS C	0.1	0.1	0.92	0.92
P2	East Full Crossing	50	64.3	LOS F	0.2	0.2	0.96	0.96
P2B	East Slip/Bypass Lane Crossing	50	29.4	LOS C	0.1	0.1	0.92	0.92
P3	North Full Crossing	50	38.6	LOS D	0.1	0.1	0.92	0.92
P4	West Full Crossing	50	64.3	LOS F	0.2	0.2	0.96	0.96
P4B	West Slip/Bypass Lane Crossing	50	33.8	LOS D	0.1	0.1	0.92	0.92
All Ped	estrians	350	46.3	LOSE			0.93	0.93

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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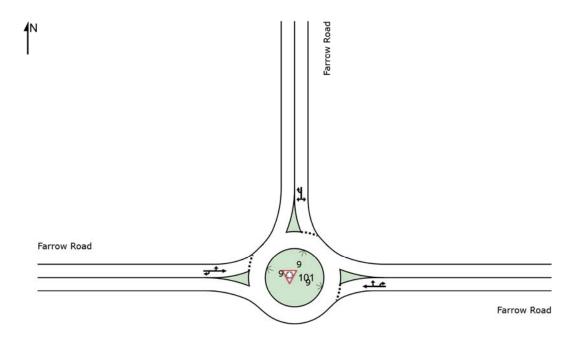
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Project: Z:\DATA\Data\Jobs01\Jobs\20work\20067Y_2FarrowRdCampbelltown\SIDRA\200304\BlaxlandRd_FarrowRd_TheKraalDr.sip8

SITE LAYOUT

Site: 101 [Existing AM]

Farrow Road Roundabout Site Category: (None) Roundabout



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Site: 101 [Existing AM]

Farrow Road Roundabout Site Category: (None) Roundabout

Movem	ent Perform	ance - Vehi	cles									
Mov ID	Turn	Deman Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	f Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Fa	rrow Road											
5	T1	21	0.0	0.152	5.6	LOSA	0.8	5.7	0.34	0.64	0.34	51.7
6	R2	111	5.4	0.152	8.8	LOSA	0.8	5.7	0.34	0.64	0.34	51.2
6u	U	34	0.0	0.152	10.3	LOSA	0.8	5.7	0.34	0.64	0.34	51.9
Approac	h	166	3.6	0.152	8.7	LOSA	0.8	5.7	0.34	0.64	0.34	51.4
North: F	arrow Road											
7	L2	482	1.7	0.433	5.0	LOSA	3.4	23.8	0.21	0.55	0.21	52.6
9	R2	57	0.0	0.433	8.1	LOSA	3.4	23.8	0.21	0.55	0.21	53.2
9u	U	93	0.0	0.433	9.6	LOSA	3.4	23.8	0.21	0.55	0.21	53.7
Approac	h	632	1.3	0.433	6.0	LOSA	3.4	23.8	0.21	0.55	0.21	52.8
West: Fa	arrow Road											
10	L2	2	50.0	0.006	7.1	LOSA	0.0	0.3	0.41	0.55	0.41	49.7
11	T1	2	50.0	0.006	7.0	LOSA	0.0	0.3	0.41	0.55	0.41	50.8
12u	U	1	0.0	0.006	10.5	LOS A	0.0	0.3	0.41	0.55	0.41	52.5
Approac	h	5	40.0	0.006	7.7	LOSA	0.0	0.3	0.41	0.55	0.41	50.7
All Vehic	eles	803	2.0	0.433	6.5	LOSA	3.4	23.8	0.24	0.57	0.24	52.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Existing PM]

Farrow Road Roundabout Site Category: (None) Roundabout

Moven	nent Perform	ance - Vehic	cles									
Mov ID	Turn	Demand Total veh/h	d Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Fa	arrow Road											
5	T1	3	0.0	0.248	4.8	LOSA	1.4	10.3	0.06	0.65	0.06	52.4
6	R2	367	1.9	0.248	7.9	LOSA	1.4	10.3	0.06	0.65	0.06	51.9
6u	U	26	0.0	0.248	9.4	LOSA	1.4	10.3	0.06	0.65	0.06	52.5
Approach		396	1.8	0.248	8.0	LOSA	1.4	10.3	0.06	0.65	0.06	52.0
North: F	arrow Road											
7	L2	126	6.3	0.102	5.0	LOSA	0.5	3.7	0.14	0.53	0.14	53.2
9	R2	2	0.0	0.102	8.0	LOSA	0.5	3.7	0.14	0.53	0.14	53.9
9u	U	4	0.0	0.102	9.6	LOSA	0.5	3.7	0.14	0.53	0.14	54.5
Approa	ch	132	6.1	0.102	5.2	LOSA	0.5	3.7	0.14	0.53	0.14	53.2
West: F	arrow Road											
10	L2	20	0.0	0.033	6.9	LOSA	0.2	1.1	0.49	0.60	0.49	52.2
11	T1	9	0.0	0.033	6.8	LOSA	0.2	1.1	0.49	0.60	0.49	53.1
12u	U	1	0.0	0.033	11.5	LOSA	0.2	1.1	0.49	0.60	0.49	53.2
Approa	ch	30	0.0	0.033	7.0	LOSA	0.2	1.1	0.49	0.60	0.49	52.5
All Vehi	cles	558	2.7	0.248	7.3	LOSA	1.4	10.3	0.10	0.62	0.10	52.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Proposed AM]

Farrow Road Roundabout Site Category: (None) Roundabout

Mov	Turn	Demand Flows		Deg.	Average	Level of	95% Back of Queue		Prop.	Effective	Aver. No.	Average
ID		Total veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate	Cycles	Speed km/h
East: Fa	arrow Road											
5	T1	21	0.0	0.181	5.6	LOSA	1.0	7.0	0.35	0.64	0.35	51.7
6	R2	145	4.1	0.181	8.8	LOSA	1.0	7.0	0.35	0.64	0.35	51.2
6u	U	34	0.0	0.181	10.3	LOSA	1.0	7.0	0.35	0.64	0.35	51.8
Approa	ch	200	3.0	0.181	8.7	LOSA	1.0	7.0	0.35	0.64	0.35	51.4
North: F	Farrow Road											
7	L2	515	1.6	0.454	5.0	LOSA	3.6	25.7	0.22	0.55	0.22	52.7
9	R2	57	0.0	0.454	8.1	LOSA	3.6	25.7	0.22	0.55	0.22	53.2
9u	U	93	0.0	0.454	9.7	LOSA	3.6	25.7	0.22	0.55	0.22	53.7
Approa	ch	665	1.2	0.454	5.9	LOSA	3.6	25.7	0.22	0.55	0.22	52.9
West: F	arrow Road											
10	L2	2	50.0	0.006	7.3	LOSA	0.0	0.3	0.44	0.56	0.44	49.6
11	T1	2	50.0	0.006	7.3	LOSA	0.0	0.3	0.44	0.56	0.44	50.6
12u	U	1	0.0	0.006	10.7	LOS A	0.0	0.3	0.44	0.56	0.44	52.4
Approa	ch	5	40.0	0.006	8.0	LOSA	0.0	0.3	0.44	0.56	0.44	50.5
All Vehi	cles	870	1.8	0.454	6.6	LOSA	3.6	25.7	0.25	0.57	0.25	52.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Proposed PM]

Farrow Road Roundabout Site Category: (None) Roundabout

Mov	Turn	Demand Flows		Deg.	Average	Level of	95% Back of Queue		Prop.	Effective	Aver. No.	Average
ID		Total veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate	Cycles	Speed km/h
East: Fa	arrow Road											
5	T1	3	0.0	0.256	4.8	LOSA	1.5	10.7	0.06	0.65	0.06	52.4
6	R2	380	1.8	0.256	7.9	LOSA	1.5	10.7	0.06	0.65	0.06	51.9
6u	U	26	0.0	0.256	9.4	LOSA	1.5	10.7	0.06	0.65	0.06	52.5
Approa	ch	409	1.7	0.256	8.0	LOSA	1.5	10.7	0.06	0.65	0.06	52.0
North: F	Farrow Road											
7	L2	178	4.5	0.138	5.0	LOSA	0.7	5.2	0.15	0.53	0.15	53.3
9	R2	2	0.0	0.138	8.0	LOSA	0.7	5.2	0.15	0.53	0.15	54.0
9u	U	4	0.0	0.138	9.6	LOSA	0.7	5.2	0.15	0.53	0.15	54.5
Approa	ch	184	4.3	0.138	5.1	LOSA	0.7	5.2	0.15	0.53	0.15	53.3
West: F	arrow Road											
10	L2	20	0.0	0.033	6.9	LOSA	0.2	1.1	0.49	0.60	0.49	52.2
11	T1	9	0.0	0.033	6.9	LOSA	0.2	1.1	0.49	0.60	0.49	53.0
12u	U	1	0.0	0.033	11.6	LOSA	0.2	1.1	0.49	0.60	0.49	53.1
Approa	ch	30	0.0	0.033	7.1	LOSA	0.2	1.1	0.49	0.60	0.49	52.5
All Vehi	icles	623	2.4	0.256	7.1	LOSA	1.5	10.7	0.11	0.61	0.11	52.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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